MGS Flood Continuous Flow Model for Stormwater Facility Design



February 6, 2002

Mel Schaefer, P.E. Bruce Barker, P.E.

MGS Engineering Consultants Inc. Olympia, WA

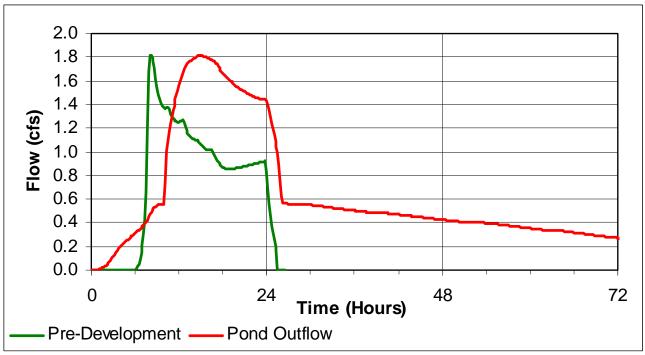
Agenda

8:00-8:30	Registration/Arrive
8:30-9:00	Stormwater Detention Standards and New State Requirements (Bruce)
9:00-9:30	Gaged Precipitation Input – Where we are Now Extended Precipitation Timeseries Input – Where we are Going (Mel)
9:30-10:00	Flood Model Overview (Bruce)
10:00-10:15	Break
10:15 -12:00	Work Session using Stormwater Model I Roadway Widening Detention Pond (Manual Design) (Bruce and Mel)
12:00-1:00	Lunch (on your own)
1:00 - 4:00	 Work Session using Stormwater Model II Roadway Widening Detention Pond (Automatic Design) Roadway Widening Infiltration Pond (Automatic Design with Manual modifications Water Quality Wet Pond Design Any Class Defined Design Problems as time Allows (Bruce and Mel)

Detention Standards and the New State Requirements

	Current Practice	New Approach	
Design Goal	Flood Control	Stream Channel Stability	
Design Standard	Peak Flow 2-year & 10-year	Match Flow Duration	
Model Type	Single Event (SCS, SBUH)	Continuous (MGSFlood, HSPF)	

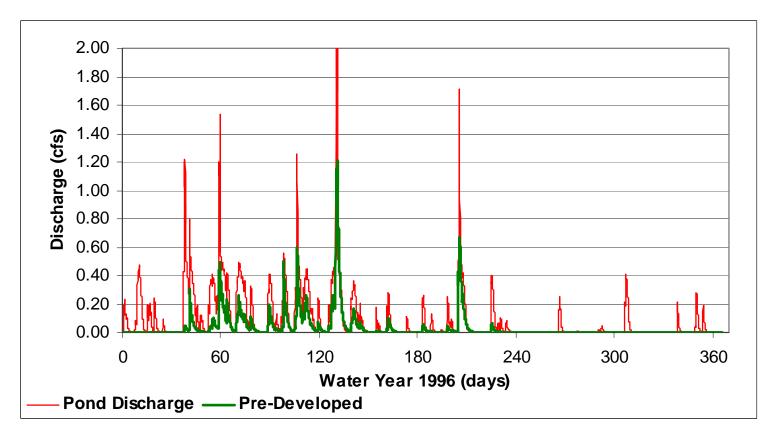
Single Event Pond Design



(Hydrographs Computed Using SBUH)

- Flood Peak is Reduced to Predeveloped Level, but higher Runoff Volume Extends Length of Flood
- Results in More Erosive Work done on Stream Channel than in Predeveloped Condition

Performance of Single Event Pond Design

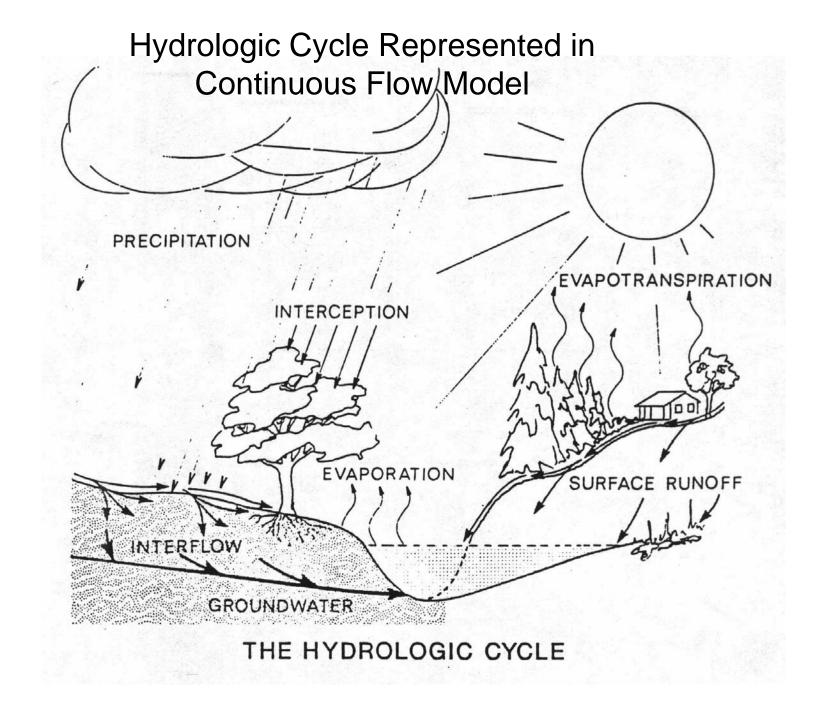


Note:

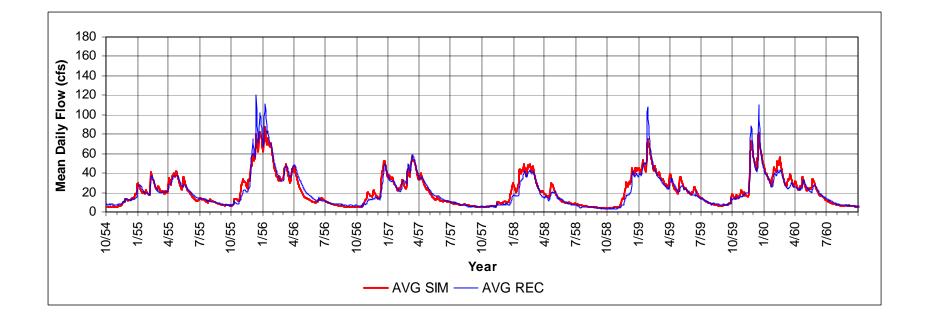
Many More Runoff Events in Postdeveloped Condition... Also Results in Greater Erosive Work on Receiving Channels

Use of Continuous Flow Model for Pond Design

- Hydrological Simulation Program FORTRAN (HSPF) is the basis for MGS Flood, KCRTS, and WWHM (HSPF http://www.epa.gov/ceampubl/ceamhome.htm)
- Simulates hourly runoff for 50 to 150 years (depending on precipitation/ evaporation record)
- Allows for pond performance to be evaluated using a wide range of storms and antecedent conditions,
- Allows for Calculation of *Flow Duration Statistics*, which are used to design ponds for Channel Stability,
- Rainfall-Runoff algorithms in HSPF are more detailed than SCS, produces much better estimates of runoff.



How Well Does HSPF do at Runoff Simulation? Example HSPF Model Calibration, Simulated and Recorded Flows Rock Creek, Cedar River Watershed, King County



Pond Design for Channel Stability:

Control the Duration of Flow to Predeveloped Levels Above the Bedload Movement Threshold

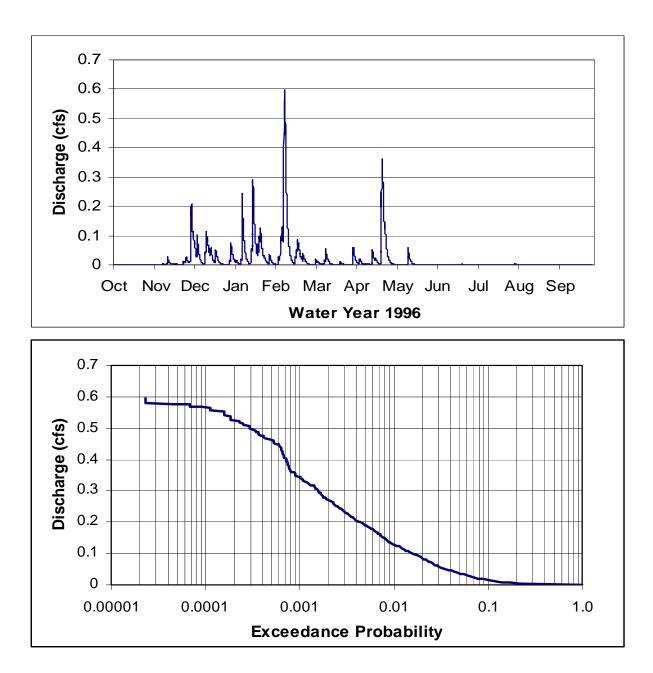
Bedload Movement Threshold:

"A rate of about 50-percent of the predevelopment 2-year discharge is a credible generic value for the initiation of sediment transport in gravel-bedded streams ..."

(Derek Booth, 2000)

Flow Duration Definition:

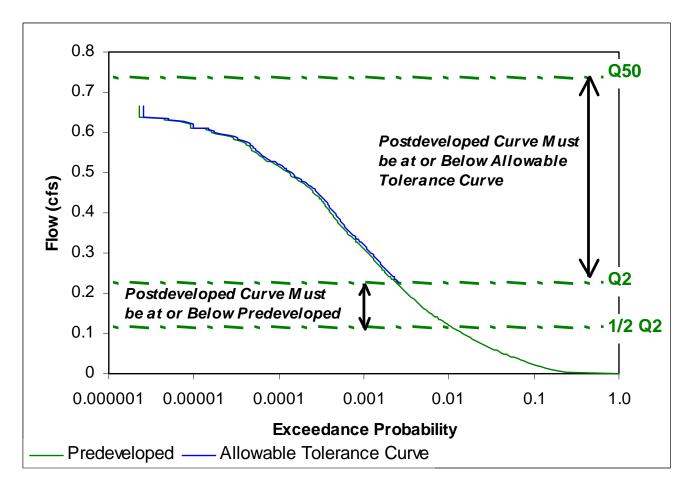
Track the Fraction of Time that a Flow is Equaled or Exceeded



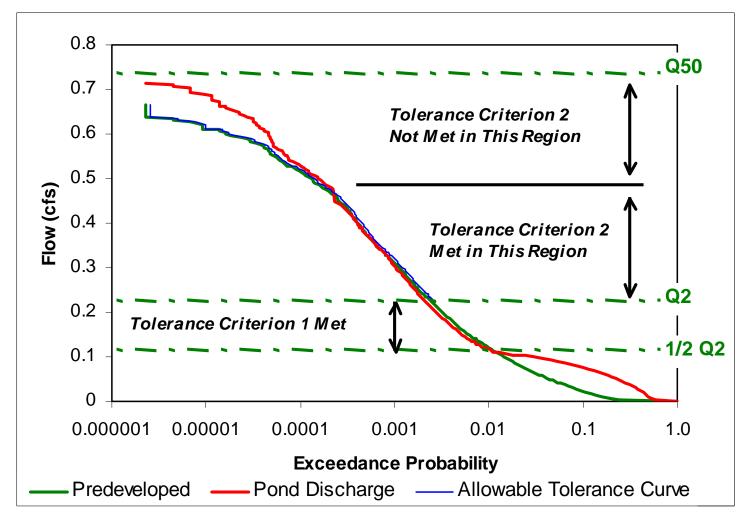
Ecology Duration Standard:

Match developed flow Durations to predeveloped durations from 50-percent of the 2-year to the full 50-year peak flow.

Ecology Duration Standard Tolerance:

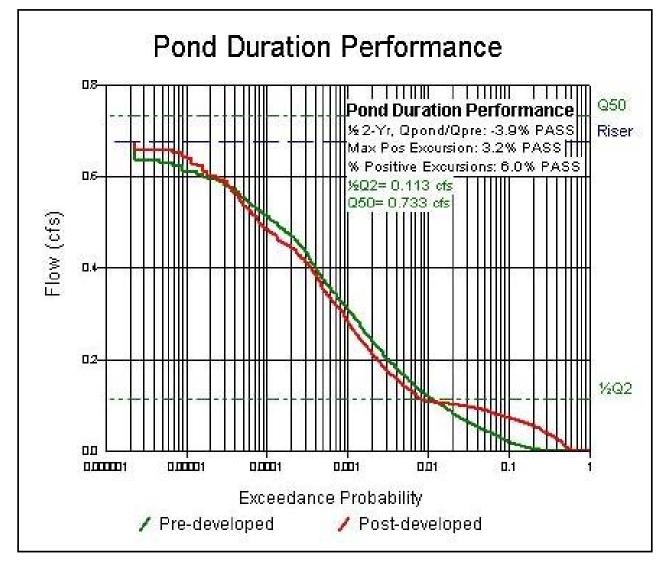


Pond Performance Example



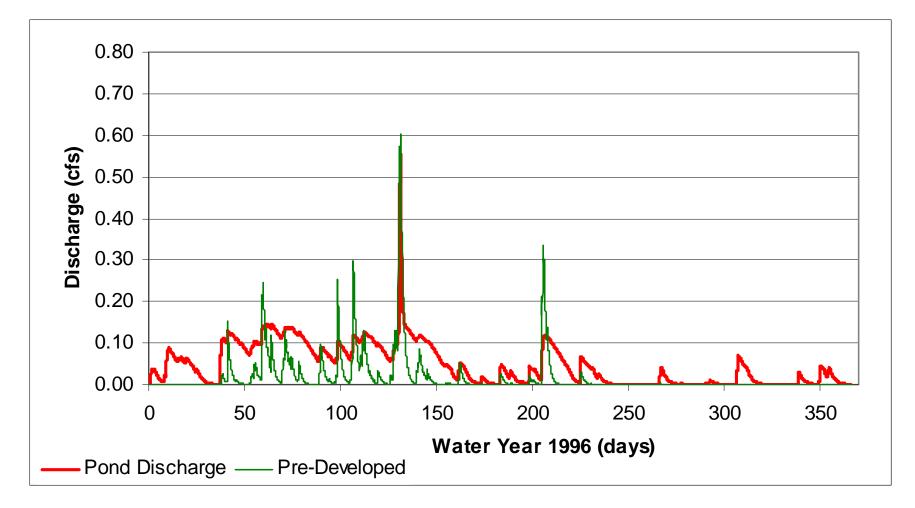
(Pond Fails Criterion 2, and Does not Meet Flow Duration Standard)

MGS Flood Pond Performance Plot



Note: Performance Criteria will be changed to Ecology's New Criteria

Performance of Duration Standard Pond



Computing Flood Recurrence Intervals

Single Event Model

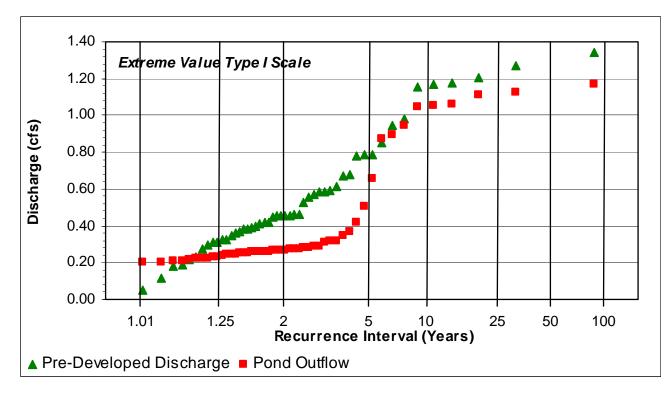
Flood Recurrence Interval Equals Precipitation Recurrence Interval (Unfortunately, this is rarely true!)

Continuous Model

- 1. Get Highest Flow Peak from Each Year of Simulation
- 2. Rank the Flows from Highest to Lowest
- 3. Assign Recurrence Interval (*Tr*) to Each Flow Using the Formula:

 $Tr = \frac{N + 0.12}{i - 0.44}$ Where: N is the total number of years simulated i is the rank of the peak flow from highest to lowest.

Flow Duration Pond, Peak Flow Performance



Note:

- Peak flow Reduced at or below predeveloped Level (Flow-Duration Ponds do a Good Job at Flood Control),
- \succ 1/2 of Data Lies Below the 2-Year
- Few Data points beyond the 10-year (because of record length),

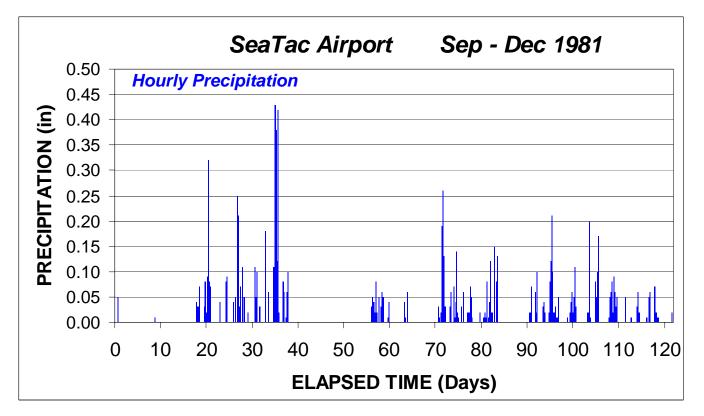
Use of Precipitation Time-Series in Continuous Hydrological Modeling

Past/Common Practice
 use of nearest precipitation gage

New/Future Practice
 extended precipitation time-series



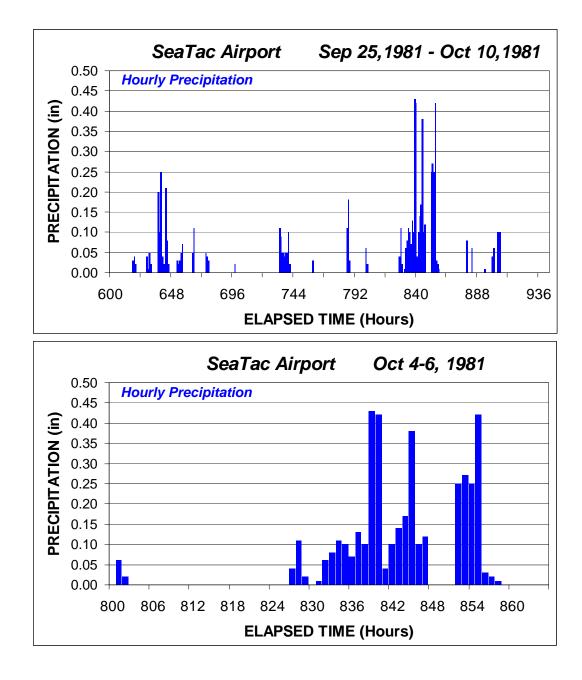
Hourly Precipitation Time-Series

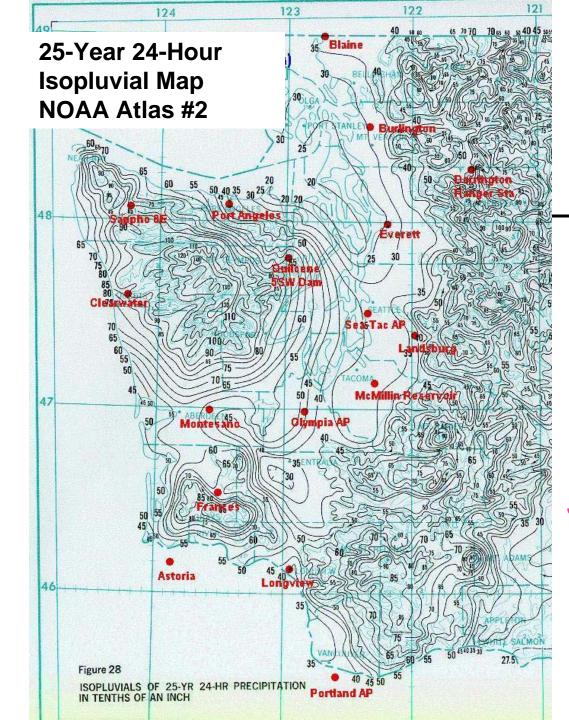


Sequence of Hourly Precipitation



Hourly Precipitation Time-Series



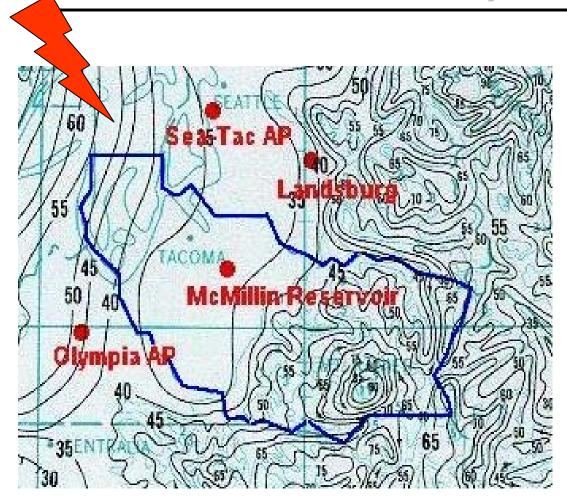


Selection of Precipitation Time-Series Common Practice

Use Nearest Hourly Gage Multiply All Hourly Values by Single Scaling Factor

Scaling Factor is Ratio of 25-Year 24-Hour Precipitation at Site of Interest Relative to Gage

Selection of Precipitation Time-Series Example



Site of Interest in Kitsap County 5.1-inches 25-Year 24-Hour

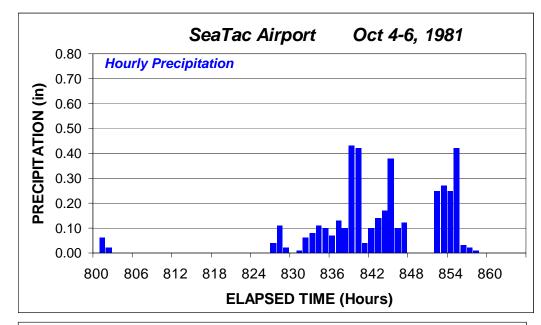
Use Sea-Tac Gage 3.0-inches 25-Year 24-Hour

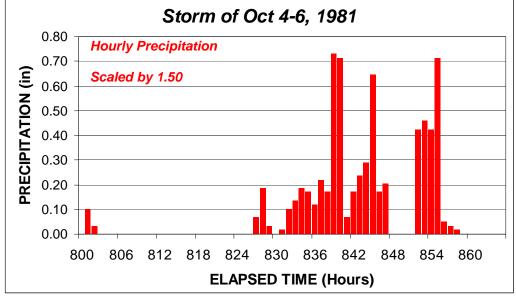
Scaling Ratio = 1.70 (5.1/3.0)

25-Year 24-Hour Isopluvial Map - NOAA Atlas #2

<u>Simple</u> Scaling of Hourly Precipitation Time-Series

Storm Scaled by 1.7 for Kitsap County Site





Selection of Precipitation Time-Series Common Practice of Simple Scaling

Shortcomings

1. Nearest Gage May Not have "Representative" Record

By chance - via Mother Nature Record may be an "active record" with one or more extreme storm events (outliers) ()r Record may be a "benign record" with the absence of many noteworthy storms And/Or Record may be of poor quality - missing data and machine malfunctions

Selection of Precipitation Time-Series Common Practice of Simple Scaling

Shortcomings

2. Storm Characteristics Vary by Duration and Season

Not Possible to Rescale Time-Series with Single Scaling Factor and Obtain Correct Storm Characteristics at all Durations at the Site of Interest:

Different Scaling factors needed for range of durations: 2-hr, 6-hr, 24-hr, 3-day, 10-day, 30-day, 90-day, Annual

Selection of Precipitation Time-Series Common Practice of Simple Scaling

Shortcomings

3. Many gages have short record lengths (< 40-years)

Record Length Usually Too Short for Intended Design Purposes

→ Computation of Flow-Duration Curves at 50-Year Level
→ Estimation of 100-Year Flood

Solution to Shortcomings of Simple Scaling

Create Extended Precipitation Time-Series

Grew out of basic need for:

robust statistical method for transposing time-series from one site to another



Extended Precipitation Time-Series

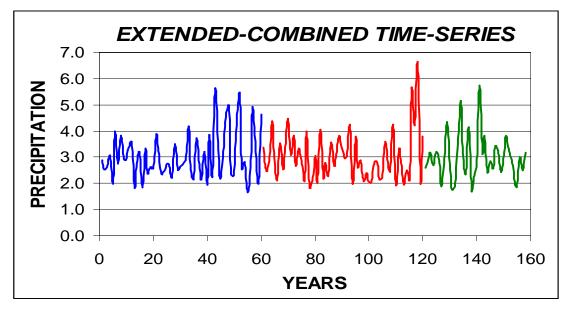
- WHAT is extended time-series record
- WHY use extended time-series record
- HOW were extended time-series developed



What is an Extended Precipitation Time-Series

Long Precipitation Record

Obtaining by Combining Records from Distant Stations



Record from Each Station Rescaled to have Storm Statistics Representative of Site of Interest

What is an Extended Precipitation Time-Series

Long Time-Series Created by Combining Precipitation Records

Vancouver, BC	38-years
Seattle, WA	60-years
Salem, OR	60-years

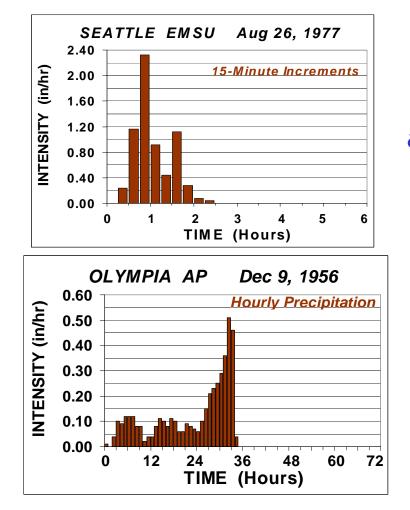
Why use Extended Precipitation Time-Series

• Allows use of high-quality stations with long records

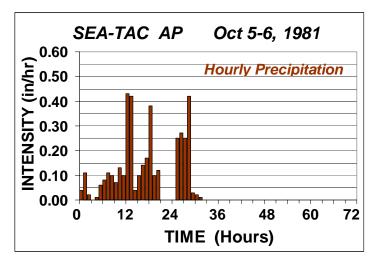
• Avoids pot-luck of using nearby stations Many hourly stations have short records of poor-quality (missing data)

- Provides greater diversity and variability of storm temporal patterns
- Provides for increased number of extreme events
- Allows Interpolation of 50-year and 100-year floods rather than extrapolation

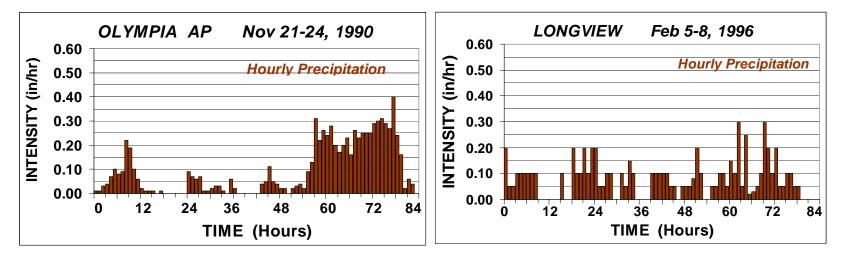
Greater Sampling of Storm Magnitudes and Temporal Patterns

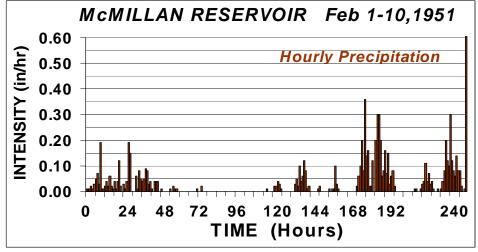


Larger Sample of Storm Temporal Patterns allows more rigorous testing of detention pond performance

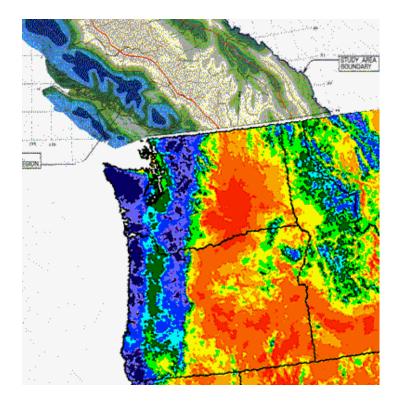


Greater Sampling of Storm Magnitudes and Temporal Patterns





HOW - Create Long Time-Series by Pooling Data from Climatologically Similar Areas



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Non-Orographic Lowlands East of Coastal Mountains

- Lowlands British Columbia
- Puget Sound Lowlands
- Willamette Valley

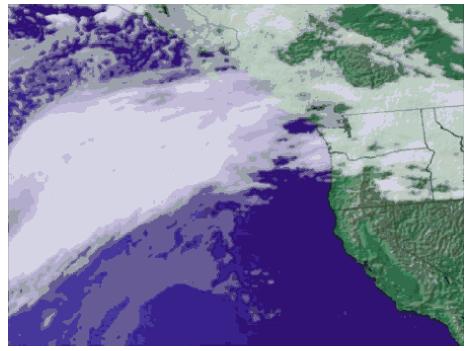
Similarity

- Seasonality of storms
- Storm temporal patterns
- Magnitude-frequency curves

HOW - Create Long Time-Series by Pooling Data from Climatologically Similar Areas

Independence of Data Allows Combining of Precipitation Records

Widely Separated Stations have Independent Records at Durations of Interest (affected by different storms)



Heaviest Precipitation Storm Tracks / Storm Centers Typically Cover Only Portion of Climatological Region MGS , Engineering Consultants, Inc.

Create Long Time-Series by Combining Precipitation Records

Stations with Hourly Records

Vancouver, BC 38-years

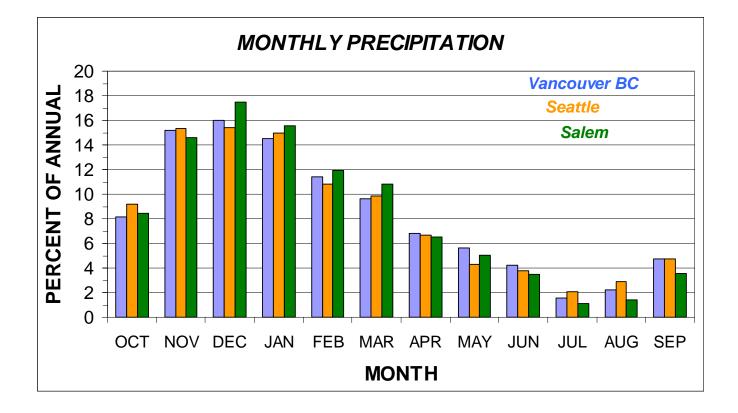
Seattle, WA 60-years

Salem, OR 60-years

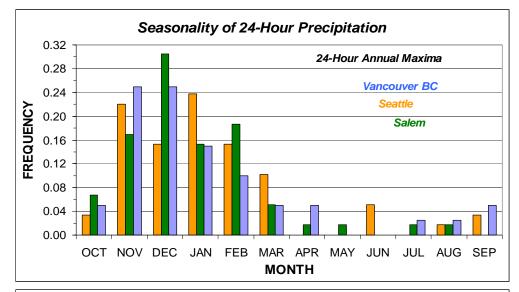
Independence of Storms at Widely Separated Stations 24-Hour Precipitation

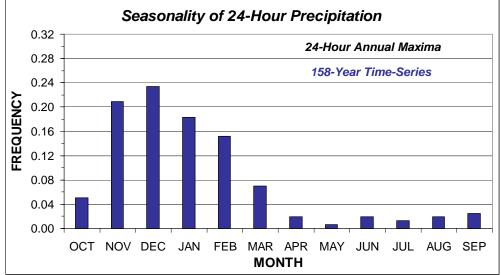
DATES OF GREATEST 24-HOUR PRECIPITATION						
RANK OF STORM	VANCOUVER, BC	SEATTLE, WA	SALEM, OR			
Greatest Precip	12 / 25 /1972	10 / 05 / 1981	11 / 18 / 1996			
2	12 / 16 / 1979	11 / 23 / 1990	10 / 26 / 1994			
3	10 / 16 / 1975	11 / 23 / 1986	02 / 16 / 1949			
4	01 / 18 / 1968	02 / 08 / 1996	03 / 30 / 1943			
5	11 / 02 / 1989	01 / 17 / 1986	12 / 02 / 1987			
6	10 / 30 / 1981	11 / 25 / 1998	01 / 20 / 1972			
7	07 / 11 / 1972	01 / 08 / 1990	02 / 05 / 1996			
8	01 / 17 / 1986	03 / 04 / 1972	02 / 09 / 1961			
9	11 / 20 / 1980	02 / 06 / 1945	01 / 03 / 1956			
10th Largest	08 / 29 / 1991	11 / 19 / 1959	01 / 14 / 1974			

Seasonal Similarity of Precipitation

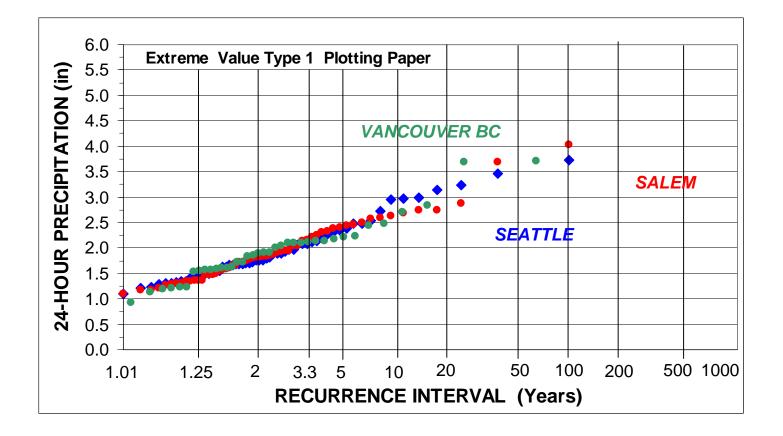


Seasonal Similarity of Precipitation





Similarity of Magnitude-Frequency Curves



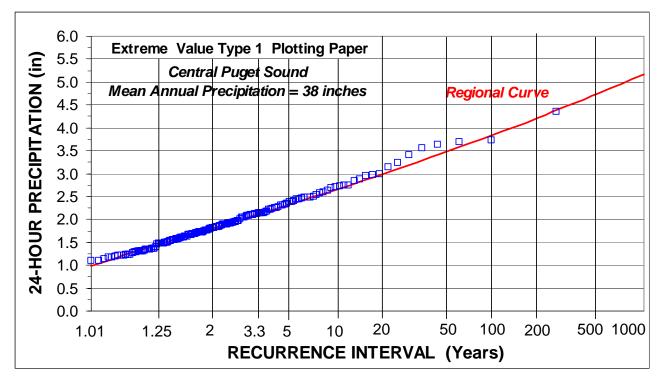
How - Extended Precipitation Time-Series

Rescale Precipitation Increments consistent with Regional Statistical Storm Characteristics for Magnitude-Frequency for: 2-hr, 6-hr, 24-hr, 72-hr, 10-day, 30-day, 90-day, Annual Durations

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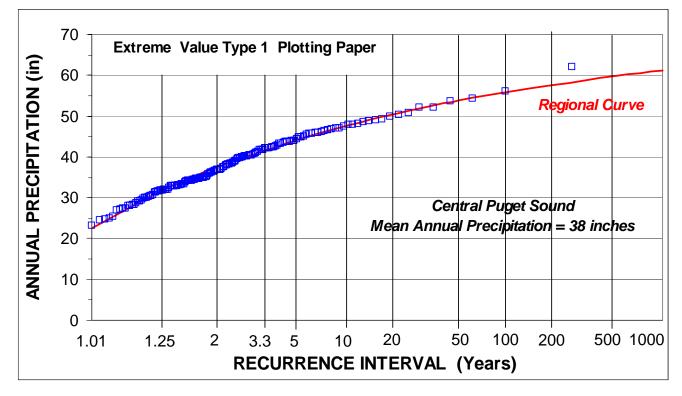
How - Extended Precipitation Time-Series

Rescale Precipitation Data based on Regional Storm Statistics

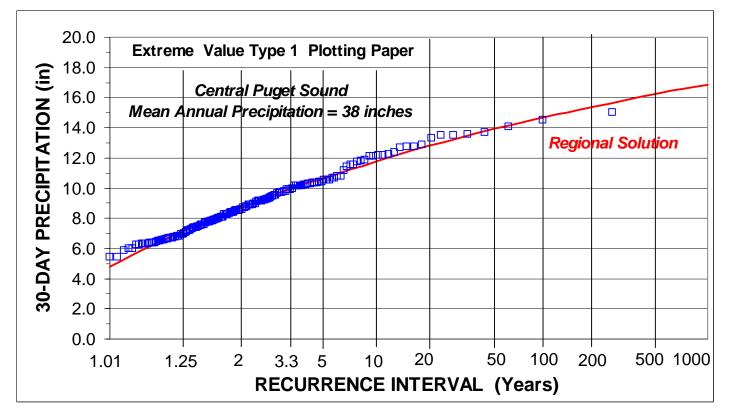


To Preserve Storm Characteristics Hourly - Daily – Weekly - Monthly - Annual

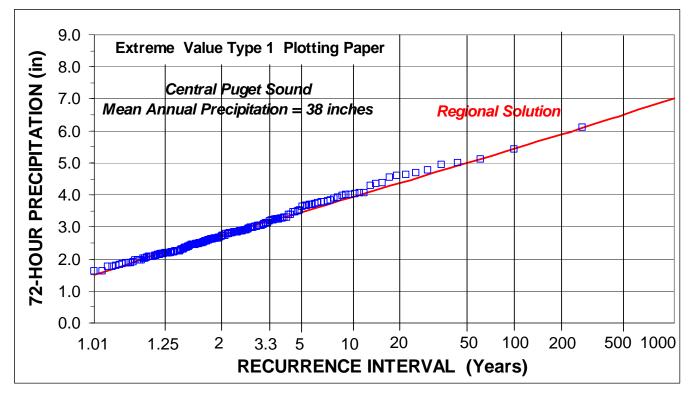
Annual Precipitation 158-Year Record



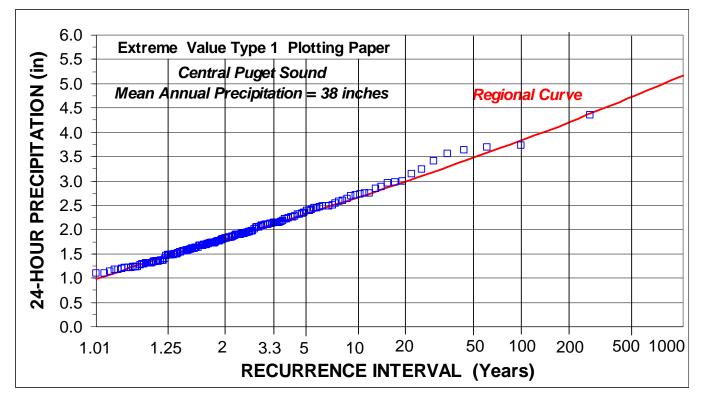
30-Day Precipitation 158-Year Record

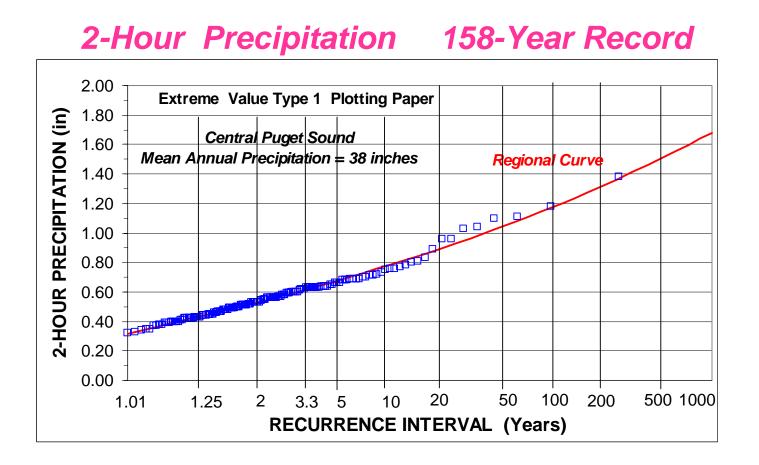


72-Hour Precipitation 158-Year Record



24-Hour Precipitation 158-Year Record





Model Deliverables Western Washington Extended Precipitation Time-Series

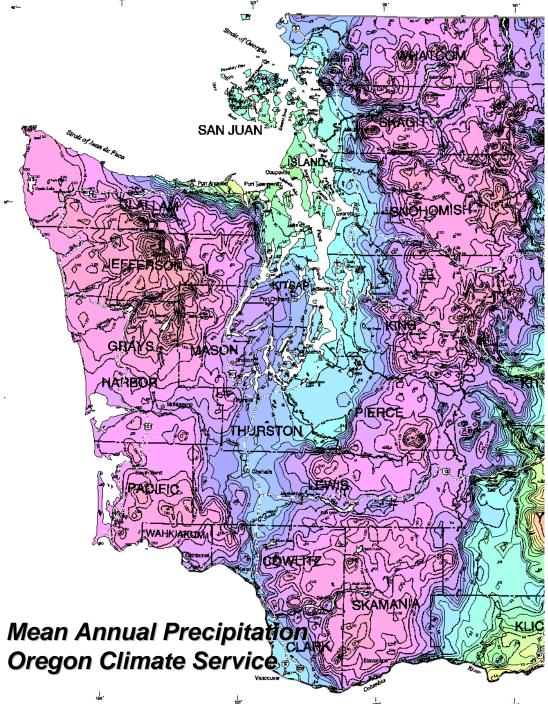
Datasets of Incremental Precipitation

Puget Sound → hourly time-series, 158-yr record

Pierce County → hourly time-series, 158-yr record

Vancouver WA Area \rightarrow hourly time-series, 121-yr record





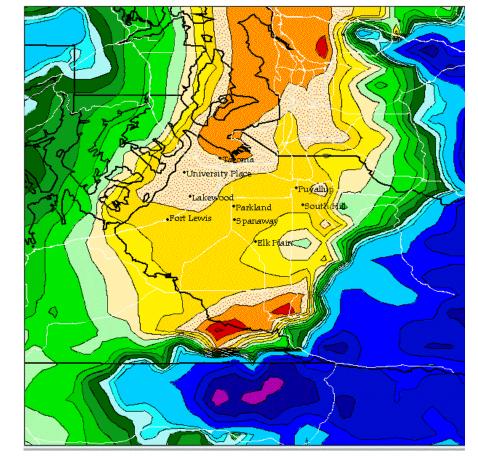
Western Washington Lowlands and Foothills

subdivided into zones of mean annual precipitation on 4-inch increments from 32 to 60-inches

> Vancouver Area 8 time-series

Puget Sound 16 time-series separate zones West and East of Central Puget Sound

PIERCE COUNTY - 15 Separate Time-Series One per 2-inch Zone of Mean Annual Precipitation



Precipitation (inches)

54 - 56

56 - 58

58 - 60

60 - 70

70 - 80

80 - 90

90 - 100

100 - 120

120 - 140 More than 140

32 - 34

34 - 36

36 - 38 38 - 40

40 - 42

42 - 44

44 - 46

46 - 48 48 - 50

50 - 52 52 - 54 38 – 52 inches Leeward / Windward Central Puget Sound

Mean annual precipitation is based on 1961-1990 averages, obtained using the PRISM model by Chris Daly of the Spatial Climate Analysis Service at Oregon State University. Data used in PRISM were collected at NOAA Cooperative stations and USDA-NRCS SNOTEL stations.

Copyright (c) 2000 by Spatial Climate Analysis Service, Oregon State University www.ocs.orst.edu/prism/prismnew.html

Use of Precipitation Time-Series in Continuous Hydrological Modeling

TRAINING TODAY

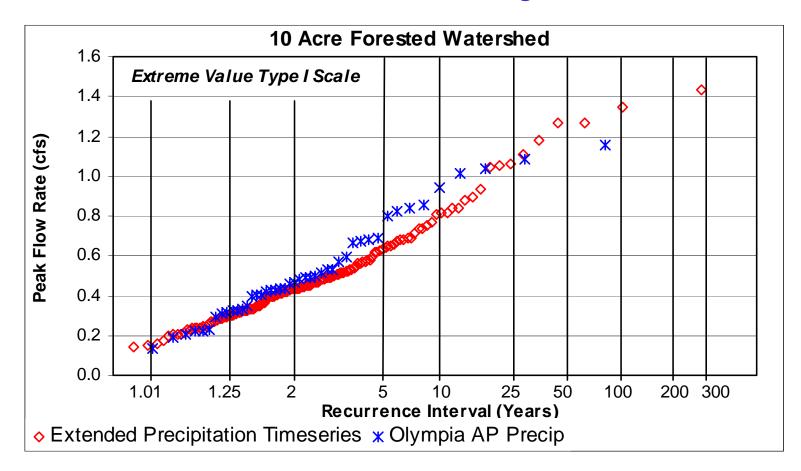
examples based on simple scaling using 24-hour 25-year precipitation

COMING IN APRIL, 2002 model delivered with extended precipitation time-series



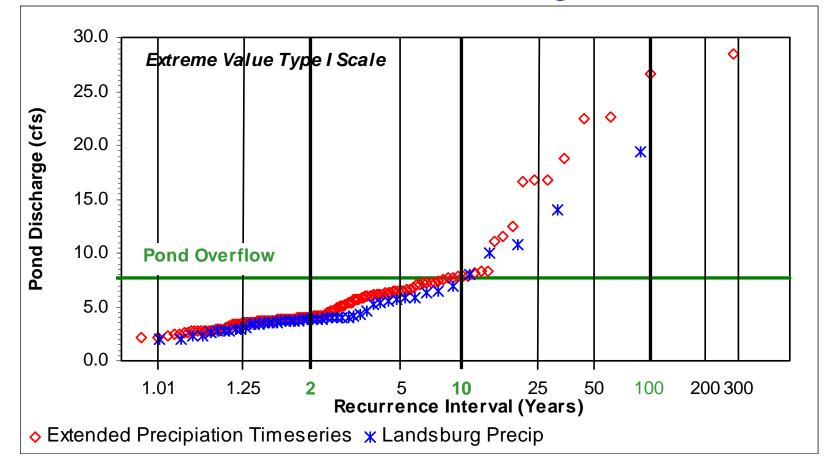
Peak Flow Comparison Forested Site

158-Year Record and 45-Year Gage Record



Peak Flow Comparison Stormwater Pond

158-Year Record and 45-Year Gage Record



MGS Flood Public Domain Version

Features:

- Meets Ecology's Stormwater Guidelines
- Uses HSPF Computational Algorithm
- Optionally Include Groundwater Discharge
- Multiple Subbasin Capability
- Can Design Facilities for "Re-development" Conditions
- Contains Statistics and Graphics Routines
- Can be Calibrated if Desired
- Final Release June 2002

MGS Flood Proprietary Version Includes Pond Hydraulics and Optimization Routines

Features:

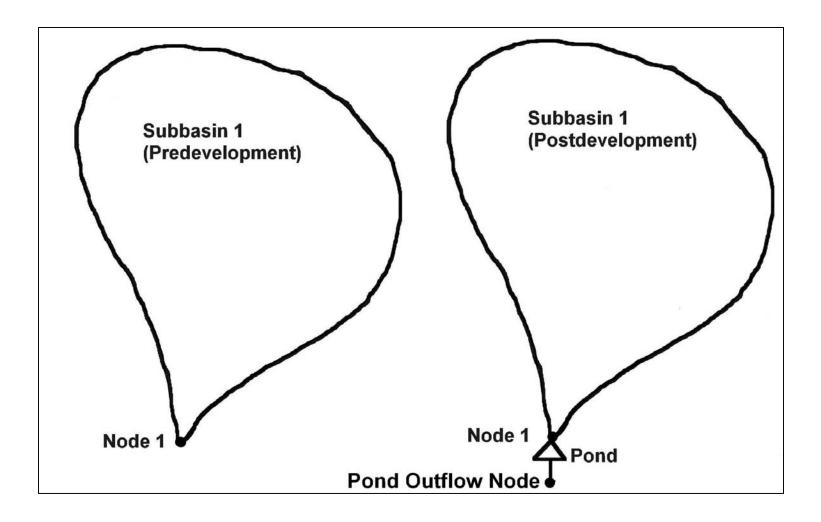
- Ability to specify Pond Geometry
- Includes a Variety of Hydraulic Structures;
 Orifice, Orifice w/ Backwater, Weirs, Risers,
 Sand Filters, Rectangular and V Notch Weirs
- Optimization Routine for Automatically Designing Ponds to Ecology Standard

Pond Design Procedure Using MGS Flood

- 1. Determine Climatic Region and 25-Yr 24-Hour Precip for Site
- 2. Enter Pre- and Post-development Land use for Each Subbasin
- 3. Assign Subbasins to "Nodes", Connect Upstream Nodes to Downstream Nodes
- 4. Compute Runoff (Saves Pre- and Post-Development Flows 50+ Years at 1-hour timestep)
- 5. Define Pond Hydraulics (either with Routing Table or Pond Hydraulics Routines)
- 6. Route Flows, Compute and Plot Duration Curves. Adjust Pond Configuration until Pre- and Postdevelopment Duration Curves Match

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Project Information Project Name Analysis Title Comments Climate Region Map Map Precipitation Station: SeaT ac Precipitation Station: Puyallup 10/01/1948 Evaporation Station: Puyallup Precipitation Scale Factor 24 Hr, 25-Yr Precip (Tenths of inch) 30.0 Precipitation Dataset Precipitation Dataset Precipitation Dataset	10/01/1999 HSPF Parameters
Project Location Watershed Layout Compute Runoff	Pond Design Tools Graphs
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MGS Flood Subbasin/Runoff Node Relationship (Simple Example)



Land Use Input Screen

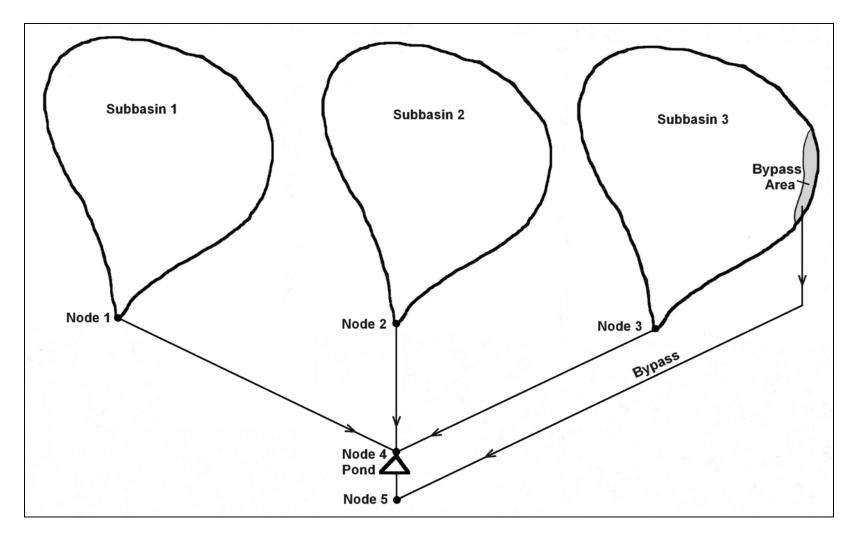
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Subbasin 1	Subbasin 2	Subbasin		obasin 4	Subbasin 5	Subbasin 6
Watershed Area (Acres)					ections	
	Predeveloped	Deve Tributary to Node	eloped By-Pass Node	Include Groundwate	er - Connect Subt	basin to Node:
Till Forest	0.000	0.000	0.000		Node 1	•
Till Pasture	0.000	0.000	0.000			
Till Grass	0.000	0.000	0.000		Connect By-P	ass Area to Node: –
Outwash Forest	0.000	0.000	0.000		None	Y
Outwash Pasture	0.000	0.000	0.000			
Outwash Grass	0.000	0.000	0.000			
Wetland	0.000	0.000	0.000			
User	0.000	0.000	0.000			
User	0.000	0.000	0.000			
User	0.000	0.000	0.000			
User	0.000	0.000	0.000			
Impervious	0.000	0.000	0.000			
Total (acres)	0.000	0.000	0.000			0k Cancel
				2/5/2002	8:53 AM	

Relationship Between SCS Hydrologic Group and Continuous Model Soil/Geologic Group

SCS Hydrologic Soil Group	MGSFlood HSPF Soil/Geologic Group
A/B	Outwash
С	Till
D	Wetland

Continuous Model Runoff Parameters were Developed by the USGS (Report No. 89-4052) based on Geology of Puget Sound Lowlands

MGS Flood Subbasin and Node Delineation, Multiple Subbasin Example with Bypass



Watershed Layout Showing Node Connections

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Subbasin Subbasin Conn	ection Summary	By-Pass Area Con		
Definitions Subbasin 1	> Node 2	Subbasin 3: By-Pass 2.00	10 Acres to Node 5	
	ode Connections - Node Connec	t to Downstream Node:		
	Node 1 Node	4 🔻		
	Node 2 Node	4		
	Node 3 Node	4 🔽		
	Node 4 None	*		
	Node 5 None			
	Node 6 None	*		
	Node 7 None	*		
	Node 8 None	*		
	Node 9 None	Y		
	Node 10 None	7		
Project Location Watershed Layout	Compute Runoff	Pond Design	Tools	Graphs
		1/23/2002	9:12 AM	li.

Runoff Computation Tab Runoff from Nodes 4 and 5 Will be Saved

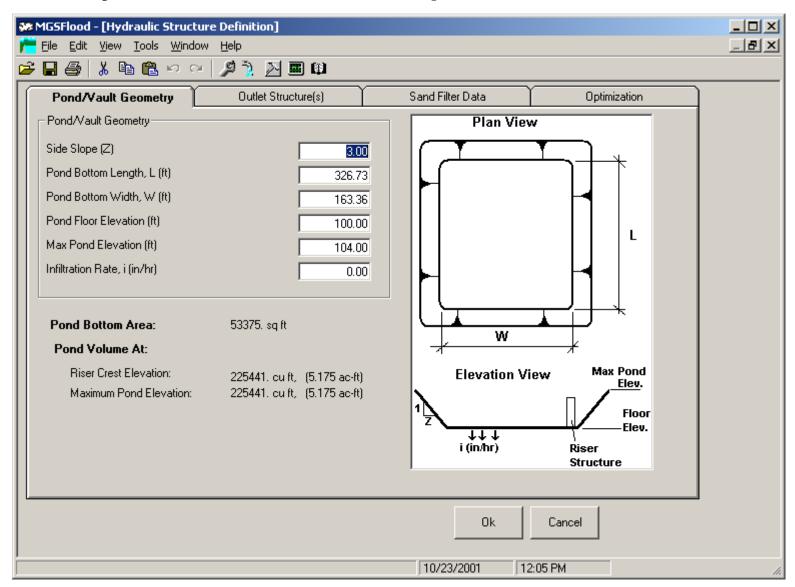
Input and Output Database Files Input: MGSRegion.mdb Precipitation Station: McMillin Evaporation Station: Puyallup Simulation Time Span File Limits Start 10/01/1948 01:00 10/01/1948 01:00 End 10/01/1996 00:00 10/01/1996 00:00 (17532 Days) Computational Timestep © 15-Minutes © 1-Hour	Save Runoff from No Node 1	des Enter Node Name	
	Hun		

Pond Design Tab, 2-Options:

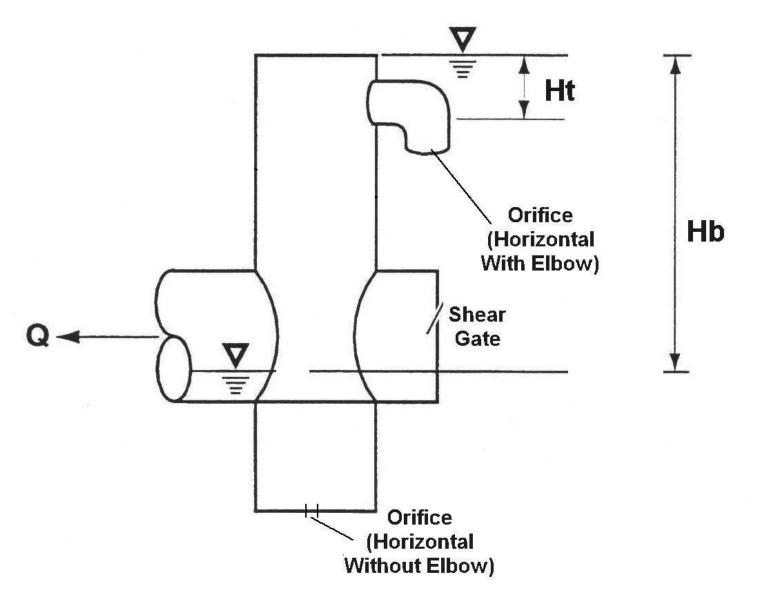
- Routing Table
- Hydraulic Structures/Optimization Routine (Proprietary)

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·	Use Hydraulic :	Structures		Hydraulic Structures		Select Inflow and Outflow Nodes for Routing Inflow Node 1: Pond Inflow
	Use Routing To Hydraulic Routi					Outflow Pond Outflow Node Period of Record
Row	Elev (ft)	Area (ac)	Vol (ac-ft)	Disch (cfs)	Infilt (cfs)	10/1/1948 1:00:00 AM - 10/1/1998
1	0.00	0.000	0.0000	0.000	0.0000	10/1/1340 1.00.00 AM - 10/1/1330
2	0.10	0.160	0.0160	0.105	0.0000	
3	0.20	0.160	0.0320	0.149	0.0000	- Route Flows
4	0.30	0.160	0.0480	0.182	0.0000	
5	0.40	0.170	0.0640	0.210	0.0000	Route
6	0.50	0.170	0.0810	0.235	0.0000	
7	0.60	0.170	0.0980	0.257	0.0000	Compute Stats, Plot Performance
8	0.70	0.170	0.1150	0.278	0.0000	Iv compute stats, Flot Feironnance
	0.80	0.170	0.1320	0.297	0.0000	
9		0.4007	0.1500	0.315	0.0000	
9 10	0.90	0.180			0.0000	
9	0.90 1.00	0.180	0.1680	0.332	0.0000	
9 10 11	-	0.180	0.1680	0.332	0.0000	
9 10 11 Wa	1.00	0.180	0.1680 of Routing (ft)	0.332	0.000	esign Tools Graphs

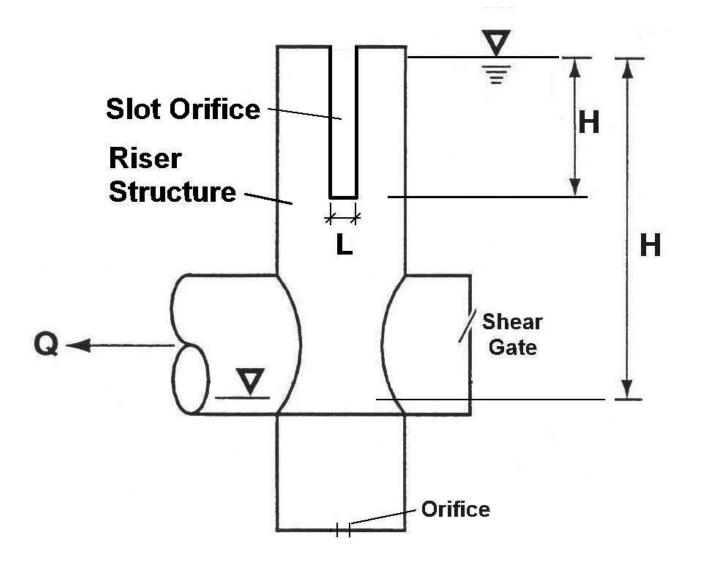
Hydraulic Structures Input Screen



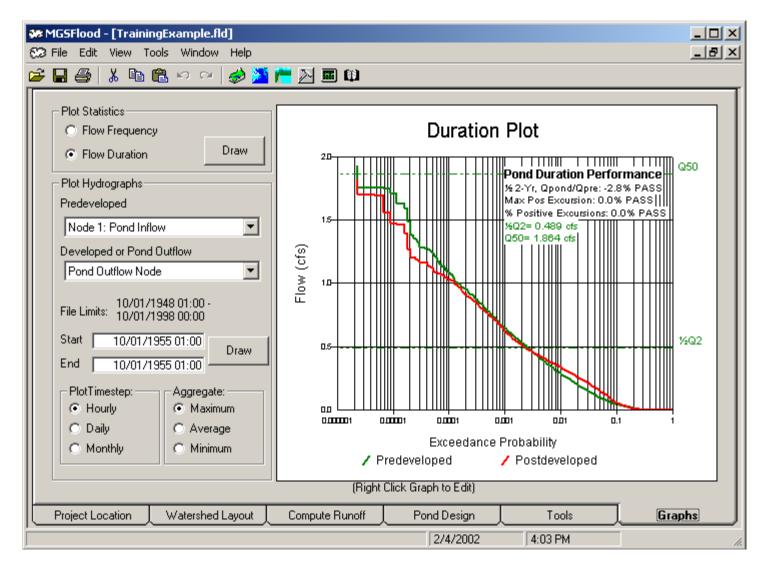
Typical Control Structure Geometry



Control Structure Geometry Configuration used by Optimization Routine



Graphs Screen (Pond Performance Plot)

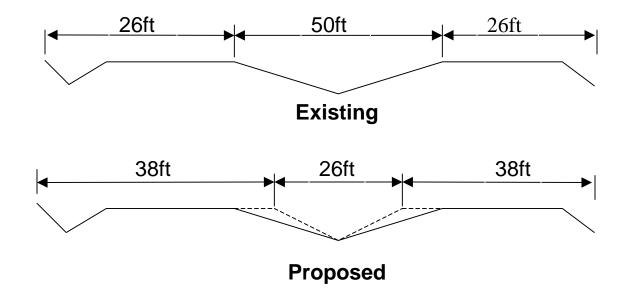






- Location: City of Des Moines, King County
- Add one lane in each direction to existing 2- lane road
- New lanes will be constructed on existing grass median
- 1 acre of off-site forest land is captured by stormdrain system

Size Stormwater Detention Pond According to Ecology's Flow Duration Standard



Some Points Regarding Detention Requirements for Roads Projects

New impervious surfaces are subject to flow control requirements if they exceed 5,000 sq. ft.

The manual requires the assumption of forest as the pre-developed condition, unless the project proponent can verify that the pre-European settlement condition was prairie.

Replaced impervious surfaces are subject to flow control if there are also new impervious surfaces on the project that will total 5,000 sq. ft. or more and total 50% or more of the existing impervious surfaces within the project limits.

 See pages 2-3 and 2-31 of Volume I of the Ecology Stormwater Manual