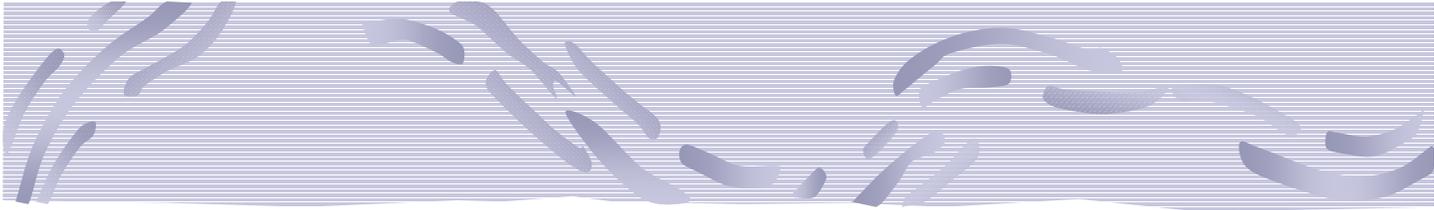


## Section 7

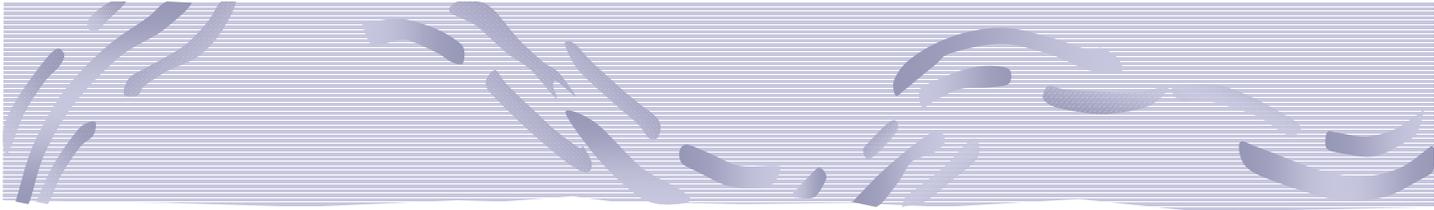
# Storm Drain System Theory





# Storm Drain Design Considerations

1. 10-yr design storm for laterals
2. 25-yr design storm for trunks
3. For pipe capacity,  $Q = VA$ , where  $V$  is velocity by Manning's equation &  $A$  is area of pipe
4. Maximum Junction Spacing  
~300' for diameter < 48"  
~500' for diameter > 48"
5. Minimum Pipe Diameter = 12" (8" for single laterals of 50' or less)
6. Full Flow Minimum Velocity = 3 ft/sec
7. Match Crowns at Junctions
8. Compute HGL if velocity > 6 ft/sec or when cover is shallow



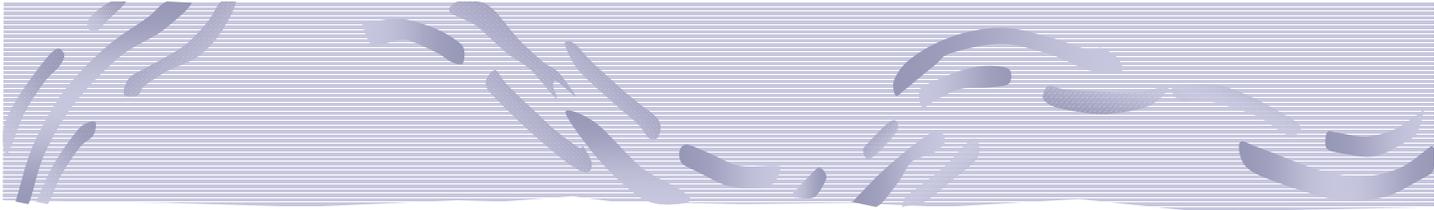
# Hydraulic Grade Line

## • Computing HGL

1. Calculate friction losses for the most downstream run of pipe.
2. Calculate minor losses for the first junction upstream of outfall.
3. Add all losses to water surface elevation at the outfall to get water surface elevation at the first upstream junction.
4. Repeat Steps 1-3.

## • MINOR LOSSES

- Friction Loss
- Entrance/Exit Loss
- Bend Loss
- Multiple Flow Loss



# Friction Loss

$$H_f = L \left[ \frac{2.15Qn}{D^{2.667}} \right]^2$$

where:

$H_f$	=	head loss due to friction in feet
$L$	=	length of pipe run in feet
$Q$	=	flow in pipe in cubic feet per second
$n$	=	Manning's roughness coefficient
$D$	=	diameter of pipe in feet

# Entrance/Exit Loss

$$H_e = \frac{V^2}{2g}$$

$$H_{ex} = 0.5 \frac{V^2}{2g} = \frac{V^2}{4g}$$

where:

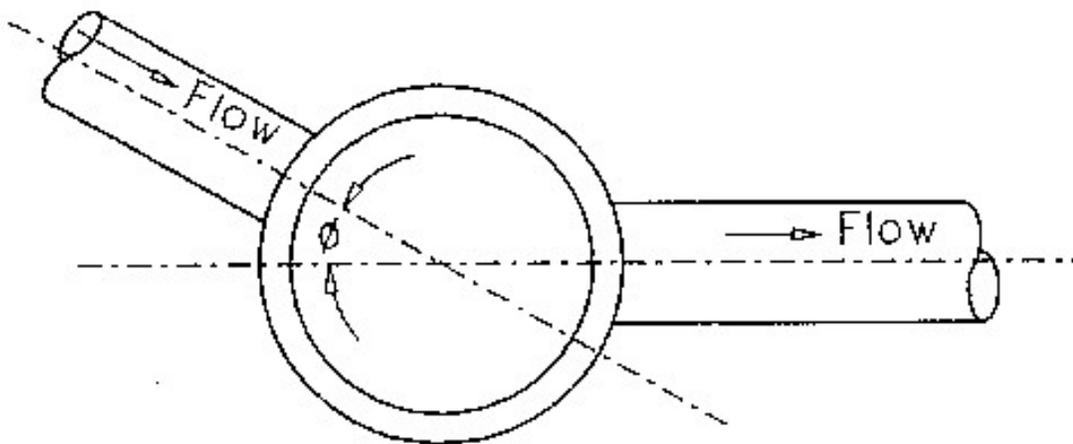
$H_e$	=	head loss from junction entrance in feet
$H_{ex}$	=	head loss from junction exit in feet
$V$	=	flow velocity in pipe in feet per second
$g$	=	gravitational acceleration constant

# Bend Loss

$$H_b = K_b \frac{V^2}{2g}$$

where:  $H_b$  = head loss from change in direction in feet  
 $K_b$  = head loss coefficient for change in direction, see below:

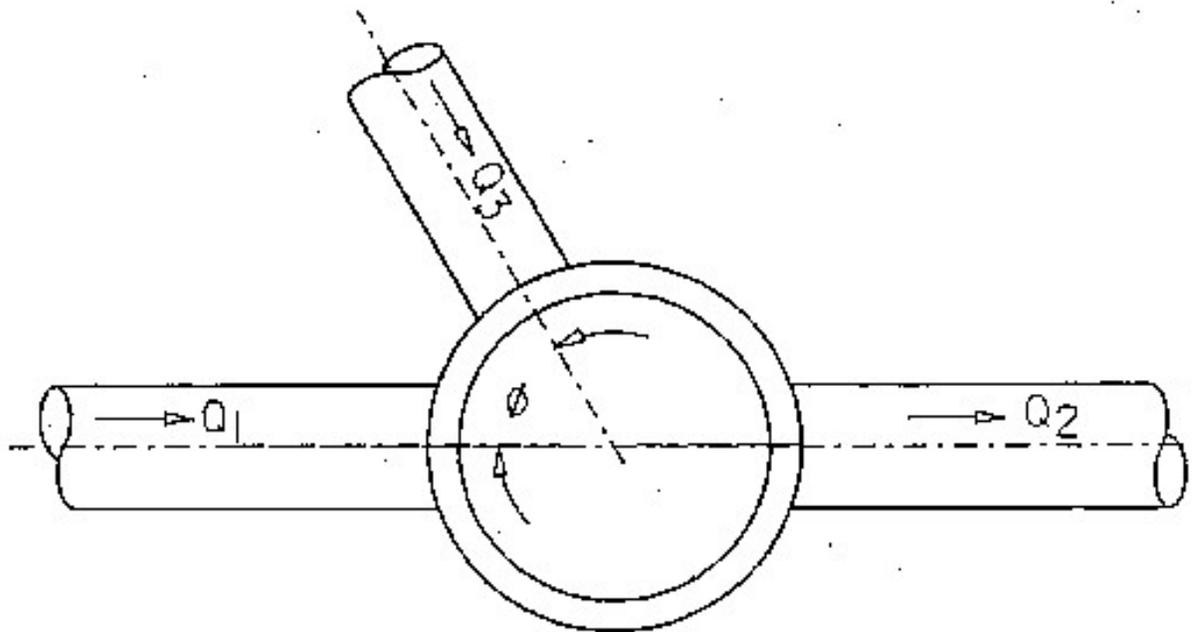
$K_b$	Angle of Change in Degrees
0.00	0
0.19	15
0.35	30
0.47	45
0.56	60
0.64	75
0.70	90 and greater



# Multiple Flow Loss

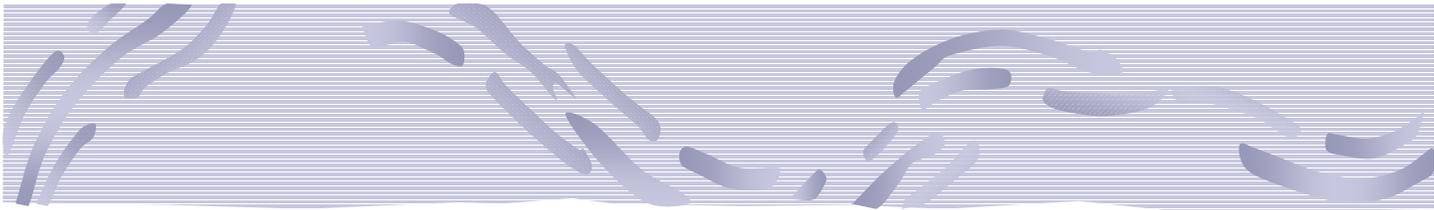
$$H_m = \frac{Q_2 V_2^2 - Q_3 V_3^2 - Q_1 V_1^2 + K_b Q_3 V_3^2}{2gQ_2}$$

where:  $H_m$  = head loss from **multiple flows** in feet  
 $K_b$  = head loss coefficient for **change in direction**



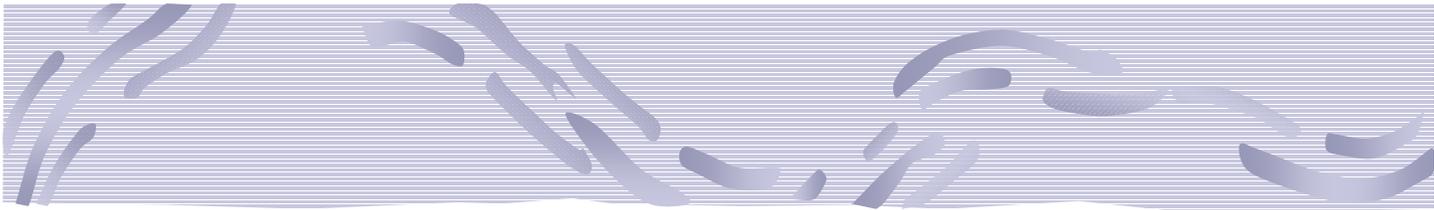






Calculated By			Date			Checked By			Date			
Location			Discharge									
Ditch Located On	From Station	To Station	Source Of Drainage	Drainage Area (A)	Runoff Coeff. (C)	CA	ICA	T <sub>c</sub> To Col. 2 Station	Rainfall Intensity	Runoff	Con. Inflow	Total Flow
1	2	3	4	5	6	7	8	9	10	11	12	13





**Storm Drain Design Calculations**

SR		Project				Sheet #		or		Sheets	
Drain Design				Drain Profile							Remarks
Pipe Diameter	Pipe Slope	Flow Velocity	Pipe Capacity	Length	Elev. Change	Upstr. Ground Elev	Downstr. Ground Elev.	Upstr. Invert Elev.	Downstr. Invert Elev.		
14	15	16	17	18	19	20	21	22	23	24	

