

**APPENDIX C**  
**Geophysical Surveys**



**APPENDIX C.1**  
**Report of Geophysical Survey - Slide Curve Bridge and Walls**





May 20, 2008

Mr. Brian Hamilton, P.G.  
Manager, Transportation and Tunneling  
URS Corporation  
1501 4th Avenue, Suite 1400  
Seattle, Washington 98101

**Subject:       Geophysical Investigation Report  
                  I-90 Snoqualmie Pass East Project  
                  Snoqualmie Pass, Washington**

Dear Mr. Hamilton:

URS Corporation (URS) is pleased to present the findings of a geophysical investigation conducted as part of the Washington State Department of Transportation I-90 improvement project near Snoqualmie Pass, Washington. The objective of the geophysical investigation was to evaluate the depth to bedrock along the alignment of a structural retaining wall. The investigation consisted of a seismic refraction survey.

### **Background**

The I-90 Snoqualmie Pass East project is located along the existing Interstate I-90 between the Hyak interchange and the West Easton interchange in Kittitas County. This investigation was conducted to evaluate conditions in the area of proposed Structural Wall 1 (SW1) shown on Figure 1.

### **Seismic Refraction Methodology**

The seismic refraction method consists of transmitting seismic energy into the ground and recording the arrival of the direct and refracted compressional-waves (P-waves) at preset distances along the ground surface. Seismic energy travels through each subsurface layer with a characteristic P-wave velocity that is dependent on the density, compressibility, pore space, and fluid content of the geologic material. By evaluating seismic velocities, as inferred from the recorded first arrival travel times, and the seismic velocity contrasts, the investigator can interpret the configuration and depths of the subsurface geologic units.

A limitation of the seismic refraction method is that the refracting layer must be sufficiently thick to be detected. The thickness required for detection depends on layer depth, velocity contrast with the overlying and underlying layer, and field parameters utilized during data acquisition and recording. A second limitation is related to the required assumption that seismic velocities increase with depth. That is, the method can detect a relatively hard layer (with a corresponding relatively high seismic velocity) under a relatively soft layer (with a corresponding relatively low seismic velocity). However, the method does not provide a means of determining the depth to, and seismic velocity of, a lower velocity layer underlying a higher velocity layer.

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A seismic refraction data collection system includes a seismograph, geophones, cables, and a seismic energy source. The seismic source can be generated using a sledgehammer, a dropped or propelled weight, explosives, or other specialized device. Selection of an appropriate seismic source for a particular study area involves consideration of the required depth of investigation, background noise levels, and the attenuation properties of the near-surface materials.

Seismic refraction surveying is conducted in straight lines or profiles. The geophones are coupled to the ground surface at predetermined distances along the line and connected via data cable to a seismograph. The seismic waves produced by the by the seismic source are sensed by the geophones and translated into an electrical signal. These data are recorded by the seismograph and saved as digital files.

The seismic data can be processed and analyzed using seismic refraction interpretation software programs. The software programs facilitate application of topographic corrections, construction of time-distance plots, determination of apparent layer velocities, and calculation of layer depths and thicknesses. Interpreted results are presented as cross-sections, i.e., velocity sections, depicting depth, thickness, and velocity of the various subsurface layers encountered.

### **Site Geology**

The information regarding site geology was taken from URS reports relevant to the I-90 project from 2006 and 2007. The Project area geology consists of volcanic and sedimentary rocks of the Ohanapecosh and Naches Formations, which are Oligocene to Eocene in age (Tabor et al., 2000). The southern area is underlain by the tuff member of Keechelus Lake of the Ohanapecosh Formation and, to a lesser extent, andesite and basalt of the Ohanapecosh Formation. The tuff member of Keechelus Lake is a dacite crystal-vitric tuff and tuff breccia. The northern area is underlain by sedimentary and volcanic rocks (andesite and/or basalt) of the Naches Formation. Late Pleistocene alpine glacial deposits, and post-glacial Holocene alluvium, colluvium, landslide, and lacustrine deposits of Keechelus Lake are locally present.

### **Seismic Refraction Field Investigation**

The field investigation was completed between April 8 and April 10, 2008, by representatives of the URS Geophysics Group, based in Gaithersburg, Maryland. The seismic refraction survey was conducted using a Geometrics 24 Channel Geode seismograph, with 24 8-Hz P-wave geophones. The geophones were placed along the seismic lines at uniform increments of 10 feet. The seismic pulse was generated by firing a percussion Betsy® Seisgun source at designated shot point locations (shot points). Each seismic spread contained seven shot points.

The Betsy Seisgun is a modified 21-millimeter (8-gauge) industrial shotgun mounted vertically on a base muffle chamber. The seismic pulse was created by manually firing 8-gauge shotgun blanks into 1 foot-deep holes. The number of firings at each shot point ranged from 1 to 3, depending on observed signal quality. The process of collecting multiple firings at each location is referred to as signal stacking and is intended to enhance the overall signal-to-noise ratio, thereby allowing more precise detection of first arrivals. At several shot points, impact of a 10-

pound sledgehammer against a metal plate was used instead of or in combination with the Seisgun to provide the seismic source.

Data were collected along a single line consisting of six seismic spreads. The location of the line was determined by on-site URS project representatives. The original location of the seismic line was positioned to follow the alignment of the structural wall SW1. However, based on safety concerns associated with the unstable surface materials along the steep slope, partial submersion of the wall alignment under Lake Keechelus at the time of the survey, and poor coupling of the geophones to the blocky fill surface materials, the location of the seismic line was adjusted to the right shoulder of the existing eastbound I-90. The line extended along a narrow strip between the jersey barriers and the edge of the embankment above Lake Keechelus as shown on Figure 1. The survey consisted of six adjoining 230 feet long spreads for a total of 1,380 linear feet of surveying. It should be noted that the locations of the off-end shot points are not depicted on Figure 1.

### **Data Processing and Analysis**

The seismic refraction data were analyzed using the software package SeisImager from Geometrics. Initial data processing steps involved setting the survey geometries, filtering noisy data, and selecting the P-wave first arrivals. Additional data processing steps involved making subsurface layer assignments, importing elevation data, and inputting the seismic data into the modeling routine in SeisImager. The elevations of the geophones and shot points were established based on analysis of topographic data on the site plans provided. Contouring and final presentation of the modeled subsurface seismic profile was completed using the program Surfer by Golden Software.

### **Seismic Refraction Results**

The results of the seismic refraction investigation are provided as Figure 2. The seismic profile extends 1,380 feet from test boring SW1-001-07 at Station 1371+50 to approximately Station 1385+17 feet. Highway traffic and concurrent drilling activity near the seismic line during data acquisition resulted in elevated background noise levels and an associated degradation of the signal quality. However, data analysis resulted in adequate signal quality to allow modeling of the refraction cross-section depicted in Figure 2.

The results generally correlate well with the depth to bedrock indicated on three borings (SW1-001-07, SW1-002-07, and SW1-005-07) located along the seismic line as shown on Figure 2. These results indicate that the contact between the gravel and the underlying andesite bedrock correlates with compressional wave velocities in the range of 6,000 to 8,000 feet/second. This velocity range is represented by the transition from the yellow to red contour interval on Figure 2. The depth to the interpreted bedrock surface ranges from about 20 feet to 72 feet but averages approximately 30 feet across the profile. The results appear to indicate a generally U-shaped area of deeper bedrock indicative of a buried valley from about profile distance 650 feet to 950 feet. This area corresponds to about STA 1378+00 to 1382+00 feet. It appears reasonable to assume that the axis of this suspected buried valley is oriented generally perpendicular to the existing slope and that it extends to the alignment of the proposed Structural Wall 1. In general one would expect for such a valley to widen in the down slope direction.

Mr. Brian Hamilton, P.G.  
November 13, 2009  
Page 4

### **Limitations**

This geophysical investigation was conducted in accordance with reasonable and accepted engineering geophysics practices, and the interpretations and conclusions are rendered in a manner consistent with other consultants in our profession. However, all geophysical techniques have some level of uncertainty and limitations. No other representations to the client is expressed or implied, and no warrant or guarantee is included or intended.

We greatly appreciate the opportunity to work with you on this project. Please feel free to contact me at (301) 258-9780 if you have any questions.

Very truly yours,  
URS Corporation



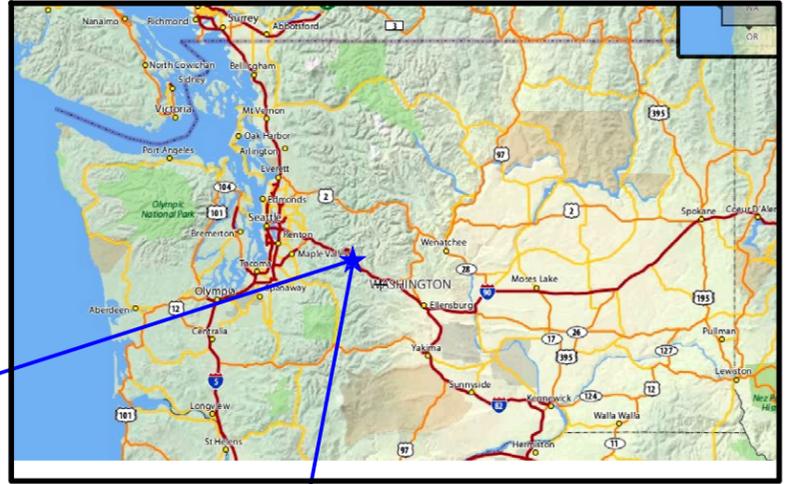
Michael R. Greer, P.G.  
Senior Geophysicist



Timothy J. King, P.G.  
Principal Geophysicist

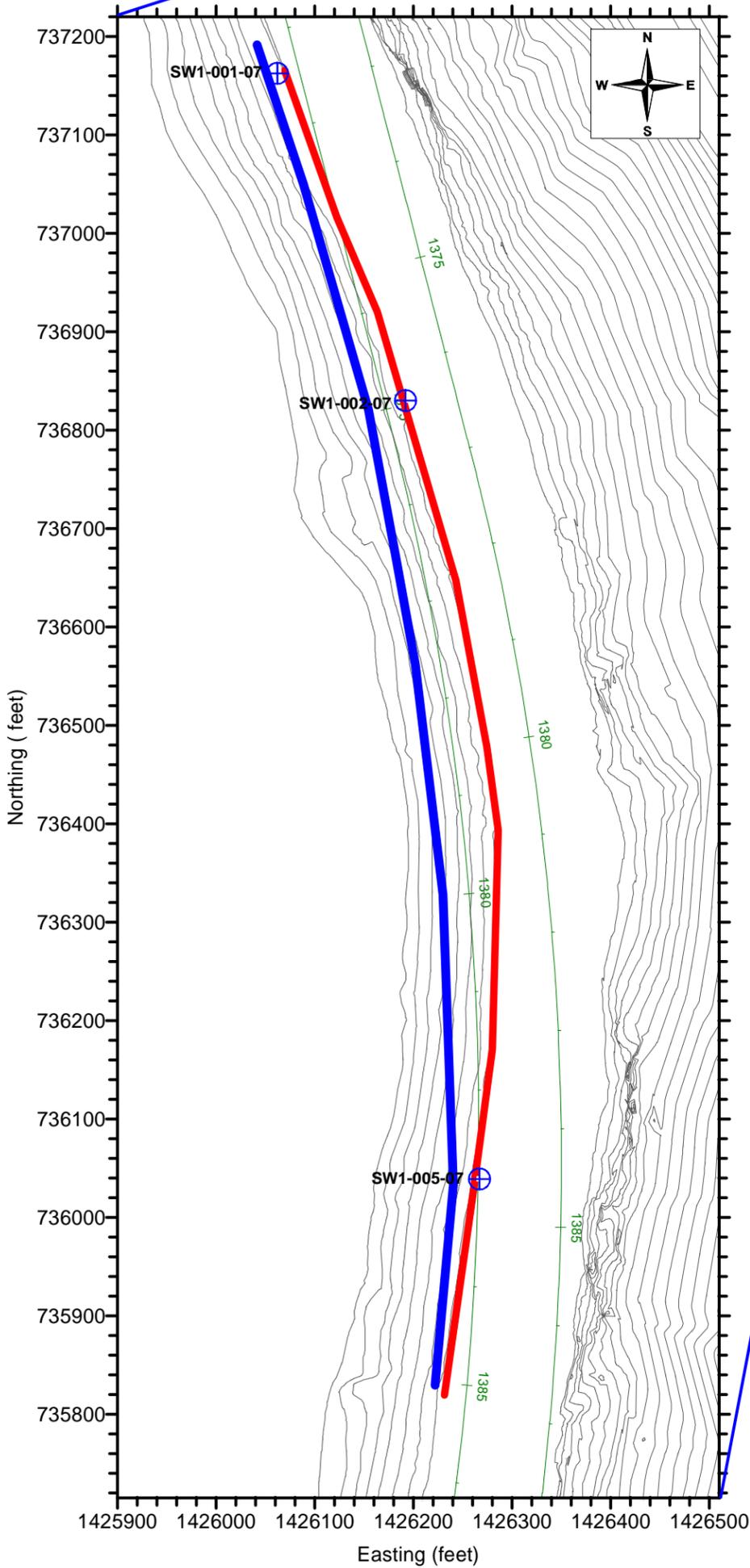
Enclosures:   Figure 1 – Site Location Map  
                  Figure 2 – Seismic Refraction Survey Results

Washington State Map



Yahoo! Local Maps

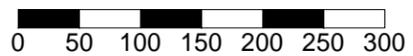
Seismic Line Map



**Legend**

-  Boring Location
-  Seismic Line 1
-  Center Line Stations
-  Sound Wall
-  Contours

Scale  
1 in. = 150 ft.



State Plane NAD-83  
Washington South

**URS** 200 Orchard Ridge Dr., Ste. 101  
Gaithersburg, MD 20878  
Geophysical Services (301) 258-9780

Geophysical Investigation  
Site Location Map

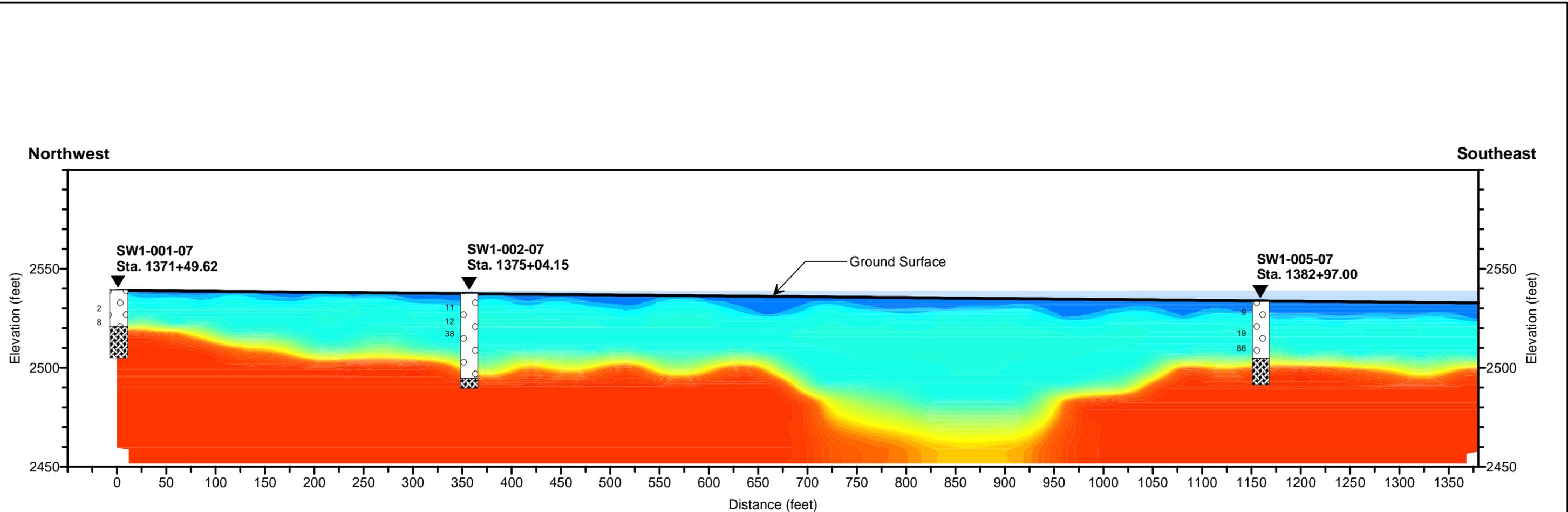
I-90 Snoqualmie Pass Structural Wall 1

Snoqualmie, Washington

DESIGNED BY	DRAWN BY	CHECKED BY	JOB NUMBER
MRG	5/15/08	MRG	5/16/08
		TJK	5/20/08

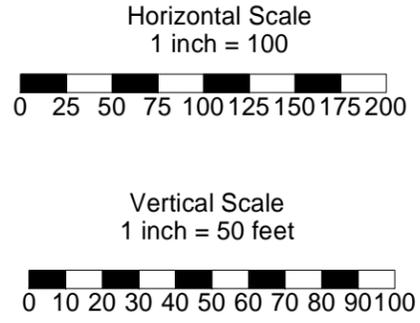
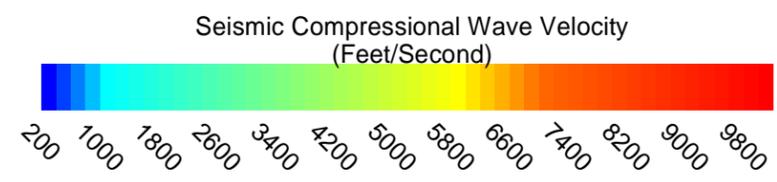
FIGURE  
1





**Legend**

Blow Counts	Description of Material
2 8	Gravel
8	Andesite



200 Orchard Ridge Dr., Ste. 101 Gaithersburg, MD 20878 (301) 258-9780			
<b>Geophysical Investigation Results</b> <b>Seismic Refraction Survey</b>			
I-90 Snoqualmie Pass Structural Wall 1			
Snoqualmie, Washington			
DESIGNED BY	DRAWN BY	CHECKED BY	JOB NUMBER
MRG	5/15/08	MRG	5/16/08
		TJK	5/20/08
			33758632
			<b>FIGURE</b> <b>2</b>



**APPENDIX C.2**  
**Report of Geophysical Survey - Slide Curve Median Wall**





**Northwest  
Geophysical  
Associates, Inc.**

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PO Box 1063, Corvallis, OR 97339-1063  
Phone: (541) 757-7231 FAX: (541) 757-7331  
www.nga.com

December 3, 2009  
NGA Ref: 717

Mr. Thomas Badger  
Washington State Department of Transportation  
State Materials Laboratory - Geotechnical Division  
PO Box 47365  
Olympia, WA 98504-7365

Re:

**DRAFT**  
***Seismic Refraction and MASW Surveys  
I-90 Snoqualmie Pass Phase 1C***

Dear Mr. Badger:

This letter report conveys the results of a pair of seismic refraction surveys and a multi-channel analysis of surface waves (MASW) survey which Northwest Geophysical Associates, Inc. (NGA) conducted for the Washington State Department of Transportation (WSDOT) as part of a geotechnical investigation for the Hyak to Keechelus Lake portion of the I-90 Snoqualmie Pass East Project (Figure 1, Site Location Map). The work was carried out under NGA's contract with WDOT for On-call Geophysical Services.

This report covers two seismic survey areas:

**Snow Shed Site:**

The Snow Shed Site is immediately south of the avalanche snow shed covering the westbound lanes of I-90. The objective of the survey was to determine depth-to-bedrock for design of the road cut along the alignment south of the snow shed. NGA employed the seismic refraction technique to acquire depth-to-bedrock. One seismic refraction profile was run along a 630 foot alignment (Figure 2, Seismic Survey Plan). The seismic line ran between WSDOT stations 1363+50 and 1369+00 for the new alignment. A description of the seismic refraction method used to conduct the investigation is attached (Attachment B).

**Slide Curve Site:**

The Slide Curve Site is at mile post 59 on I-90, approximately 3,300 feet south of the Snow Shed Site. The objective of the survey was to determine thickness of fill/slide debris in a channel feature noted in that area during the original construction of I-90.

**DRAFT: 12/3/2009 3:03 PM**

Initially a roadside multi-channel analysis of surface waves (MASW) survey was run in an attempt to use traffic noise as a surface wave source. A 600 foot MASW line was run between WSDOT stations 1403+00 and 1409+00 for the new alignment. However, insufficient coherent seismic energy was observed to allow a reliable interpretation of depth-to-bedrock. A description of the MASW technique is attached (Attachment C).

Following preliminary evaluation of the MASW data, NGA and WSDOT elected to proceed with a seismic refraction survey at that site. One seismic refraction profile was run along the same alignment.

## **SNOW SHED SITE**

### ***Field Survey***

Northwest Geophysical Associates, Inc., together with Mr. Thomas Williams of Northland Geophysical, PLLC, performed the seismic refraction survey at the Snow Shed Site on October 12-14, 2009.

Along the Snow Shed alignment, seismic refraction data were acquired on one seismic profile, composed of three seismic spreads (SL12-SL14). Those profiles and spread locations are shown on Figure 2. WSDOT boring locations are also shown on that site plan. Interpreted profile is shown in Figure 3. The profile was 630 feet in length.

The locations of the seismic lines were established by Tom Badger of WDDOT and Tom Williams during a site visit on September 2, 2009. NGA established the locations of geophones and shot points along the seismic lines in the field using 300-foot tape measures and survey flagging. The geophone locations were marked with pin flags and their positions were surveyed by a WSDOT survey crew after the completion of the seismic survey. The surveyed elevations and locations were used to develop the interpretation results for this report.

The field investigation was performed using a 24-channel Geometrics GEODE seismograph to record the data. Two sources of seismic energy were employed: Electric shotgun blanks and a 30kg slide hammer. Electric shotgun blanks used for this survey were 300 grains of black powder with an electric primer. Typically 7-10 shots were used on each line including off end shots at both ends of the spread. All seismic refraction lines used a geophone spacing of 10 feet.

### ***Data Processing & Interpretation***

Data were processed using SeisImager software from Geometrics, Inc. and the OYO Corporation. Initial three layer models were constructed and then refined using the tomographic modeling capabilities of that software. The resulting tomographic model is shown as the color grid in Figure 3. The interpreted horizons, 4,000 feet/second and 9,000 feet/second, represent smoothed contours of 4,000 feet/second and

9,000feet/second. Smoothing was done manually by the interpreter using his best professional judgment as to which small scale features were artifacts of the software inversion and which features were well supported by the data.

### ***Seismic Interpretation Results***

The interpreted results of the seismic refraction survey are shown on the interpreted profile (Figure 3). The 4,000 feet/second and 9,000 feet/second horizons represent significant changes in velocity. Elevations and location coordinates for those horizons are included as Attachment A.

We note that modeled depths from refraction are actually minimum travel path distance from the geophones to the nearest bedrock, which will not be a vertical distance in areas of steep topography, or steeply dipping bedrock. With dipping planer horizons, the distance will be perpendicular to the plane of the horizon.

### ***General Stratigraphy***

#### ***Layer 1***

Materials above the 4,000 feet/second velocity horizon on the seismic profiles (with velocities less than 4,000 feet/second) are interpreted to be overburden. Boring logs indicate this material is mostly composed of some organic material as well as silt, sand, and gravel sediments (colluvium).

#### ***Layer 2***

Below the 4,000 feet/second velocity horizon seismic velocities are greater than 4,000 feet/second. This horizon corresponds with silt, sand, and gravel materials identified on the boring logs as well as some weathered bedrock. Velocities in this transitional zone increase gradationally to the 9,000 feet/second velocity horizon.

#### ***Layer 3***

Layer 3 exhibits velocities in excess of 9,000 feet/second. This is interpreted as more competent bedrock. Boring logs indicate bedrock is meta-welded lapilli dacite tuff.

### ***Seismic Lines 12-14***

The interpretations of SL12-SL14 are shown in Figure 3. This profile showed a thin overburden, generally less than 10 feet over a transitional layer, generally less than 30 feet thick, over competent bedrock with velocities in excess of 9,000 feet/second.

### ***Boring Log Data***

Boring locations are shown on the site plan (Figure 2), and projected onto the interpreted profiles. These borings are offset 0 to 10 feet from the seismic lines. With the extreme topography the boring horizon depths or elevations are not expected to tie with the seismic horizons. However, the geology is expected to be similar.

Three borings fell within 10 feet of SL12-SL14. Boring RKS-09-07 was at seismic station 4. The boring log for RKS-09-07 showed 3 feet of overburden above meta-welded lapilli dacite tuff bedrock. Boring RKS-10-07 was at seismic station 160, offset 4 feet downslope from the seismic line. The boring log for RKS-10-07 showed 4 feet of organic and silty sand materials over meta-welded lapilli dacite tuff bedrock. Boring RKS-38-08 was at seismic station 355, offset 10 feet downslope from the seismic line. The boring log for RKS-38-08 showed 6 feet of silty sand, gravel, and cobbles over meta-welded lapilli dacite tuff bedrock. A 1 to 5 foot thick zone of loose to very-dense silty sand with gravel and cobbles at some locations was logged immediately above the bedrock interface.

## **SLIDE CURVE MASW**

### ***Field Survey***

Northwest Geophysical Associates, Inc., performed the MASW survey at the Slide Curve Site on October 15-16, 2009. Along the Slide Curve alignment, MASW data were acquired on one profile, extending 600 linear feet between WSDOT stations 1403+00 and 1409+00. This profiles and spread locations are shown on Figure 2. WSDOT boring locations are also shown on that site plan.

The location of the MASW line was established by referencing WSDOT stationing at the site. NGA established the locations of geophones and shot points along the seismic line in the field using 300-foot tape measures and survey flagging. The geophone locations were marked with paint and their positions were surveyed by a WSDOT survey crew after the completion of the seismic survey. Surveyed locations are shown in Figure 2.

### ***MASW Data Collection***

The MASW field investigation was performed using two 24-channel Geometrics GEODE seismographs to record the data. A series of 117.5 foot linear MASW arrays comprised of forty-eight 4.5 Hz geophones spaced 2.5 feet apart were used to collect MASW data along the 630 foot alignment. MASW lines/spreads were overlapped to provide a continuous velocity profile of the 630 foot alignment. Two sources of seismic energy were employed: A 30kg slide hammer and seismic surface waves caused by passing traffic.

'Active' source shots (e.g. slide hammer) were collected every 15 feet along the spread starting 30 feet off the end of the first array and ending at 630ft. Two hundred sixty-one 'passive' source data (e.g. traffic noise) records were recorded. In keeping with recently developed roadside MASW data collection procedures established by the Kansas Geological Survey, NGA collected passive roadside data that included collection times encompassing a variety of frequencies resulting from different vehicles (e.g. different gross weights, number of axels, velocities, etc.) traveling at different velocities along the MASW array. An explanation of the passive roadside MASW technique can be found at: <http://www.kgs.ku.edu/software/surfseis/roadside.html>.

### ***MASW Data Processing & Interpretation***

MASW data were processed using SurfSeis software from the Kansas Geologic Survey and SeisImager software from Geometrics, Inc. and the OYO Corporation. Overtones were constructed; dispersion curves were picked from the overtone data, and then processed together to create a 2D linear profile.

Analysis of the overtone files (dispersion curves) could not resolve the depth-to-bedrock, particularly where bedrock was less than 5-8 foot depth. Hence, NGA and WSDOT elected to proceed with the seismic refraction survey at this site.

## **SLIDE CURVE SEISMIC REFRACTION**

### ***Field Survey***

Northwest Geophysical Associates, Inc., performed the seismic refraction fieldwork November 3-4, 2009. Seismic refraction data were acquired on one seismic profile, extending 630 linear feet between WSDOT stations 1403+00 and 1409+00. The profile and spread locations are shown on Figure 2. WSDOT boring locations are also shown on that site plan. Interpreted profile is shown in Figure 4.

Surveyed position and elevation data was used from our previous MASW survey in the same location. The surveyed elevations and locations were used to develop the interpretation results for this report.

### ***Seismic Refraction Data Collection***

NGA conducted a seismic refraction field investigation along the same alignment as the MASW investigation (Figure 2) using two 24-channel Geometrics GEODE seismographs to record the data. A 40kg propelled energy generator (PEG) was used as the sole seismic source. 11 shots were used on each line including off end shots at both ends of the spread. All seismic lines used a geophone spacing of 5 feet. Seismic lines/spreads were overlapped to provide a continuous velocity profile of the 630-foot alignment.

## ***Seismic Refraction Data Processing & Interpretation***

Seismic Refraction data were processed using SeisImager software from Geometrics, Inc. and the OYO Corporation. A two-layer model was constructed. The two-layer model is shown in Figure 4. The interpreted layers, 900 feet/second velocity material and 8,900 feet/second velocity material represent an average velocity in the layers above and below the dominant refractor surface.

The interpreted results of the seismic refraction survey are shown on the interpreted profile (Figure 4). The 900 feet/second and 8,900 feet/second horizons represent significant changes in velocity. Elevations and location coordinates for those horizons are included as Attachment A.

We note that modeled depths from refraction are actually minimum travel path distance from the geophones to the nearest bedrock, which will not be a vertical distance in areas of steep topography, or steeply dipping bedrock. With dipping planer horizons, the distance will be perpendicular to the plane of the horizon.

## ***Interpreted Stratigraphy***

### ***Layer 1***

Materials in the 900 feet/second layer on the seismic profiles (with velocities near 900 feet/second) are interpreted to be fill and/or slide debris. Boring logs indicate this material is mostly comprised of gravel and some silt.

### ***Layer 2***

Layer 2 exhibited a range of high velocities, best represented by 8,900 feet/second. This layer corresponds with andesite and lapilli tuff units identified on the boring logs.

## ***Seismic Lines 15-17***

The interpretations of seismic lines 15-17 are shown in Figure 4. This profile showed a varying thickness wedge of material, becoming no deeper than 25 feet, over competent bedrock with velocities near 8,900 feet/second.

Two borings fell on seismic lines 15-17. SW2-001 is located at seismic station 445. The boring log for SW2-001 showed 10.5 feet of overburden above andesite bedrock. Boring SW2-003 is located at seismic station 610. The boring log for SW2-003 showed 1.5 feet of overburden over lapilli tuff bedrock.

### ***Boring Log Data***

Boring locations are shown on the Site Plan, Figure 2, and projected onto the interpreted profiles. Contacts indicated at depth in the boring logs correlate well with the interpreted interface for seismic velocity layers 1 and 2.

### **CLOSURE**

Northwest Geophysical Associates, Inc. performed this work in a manner consistent with the level of skill ordinarily exercised by members of the profession currently practicing under similar conditions. No warranty, express or implied, beyond exercise of reasonable care and professional diligence, is made. This report is intended for use only in accordance with the purposes of the study described within.

Please feel free to contact us if you have any questions or comments regarding this information, or if you require further assistance. We appreciated the opportunity to work with you on this project and look forward to providing you with geophysical services in the future.

Sincerely,

**Northwest Geophysical Associates, Inc.**

Rowland B. French, L.G.  
President

Neil T. McKay  
Project Geophysicist

Figures 1-4  
Attachments A-C

File: NGA Snoqualmie Pass Phase 1C Rpt03.doc  
NGA Project: 717

## **LIST OF FIGURES**

- Figure 1. Site Location Map
- Figure 2. Seismic Survey Plan
- Figure 3. Seismic Line 12-14
- Figure 4. Seismic Line 15-17

## **LIST OF ATTACHMENTS**

- Attachment A: Seismic Depth Interpretation Spreadsheet
- Attachment B: Seismic Refraction
- Attachment C: MASW



FIGURE 1

SITE LOCATION

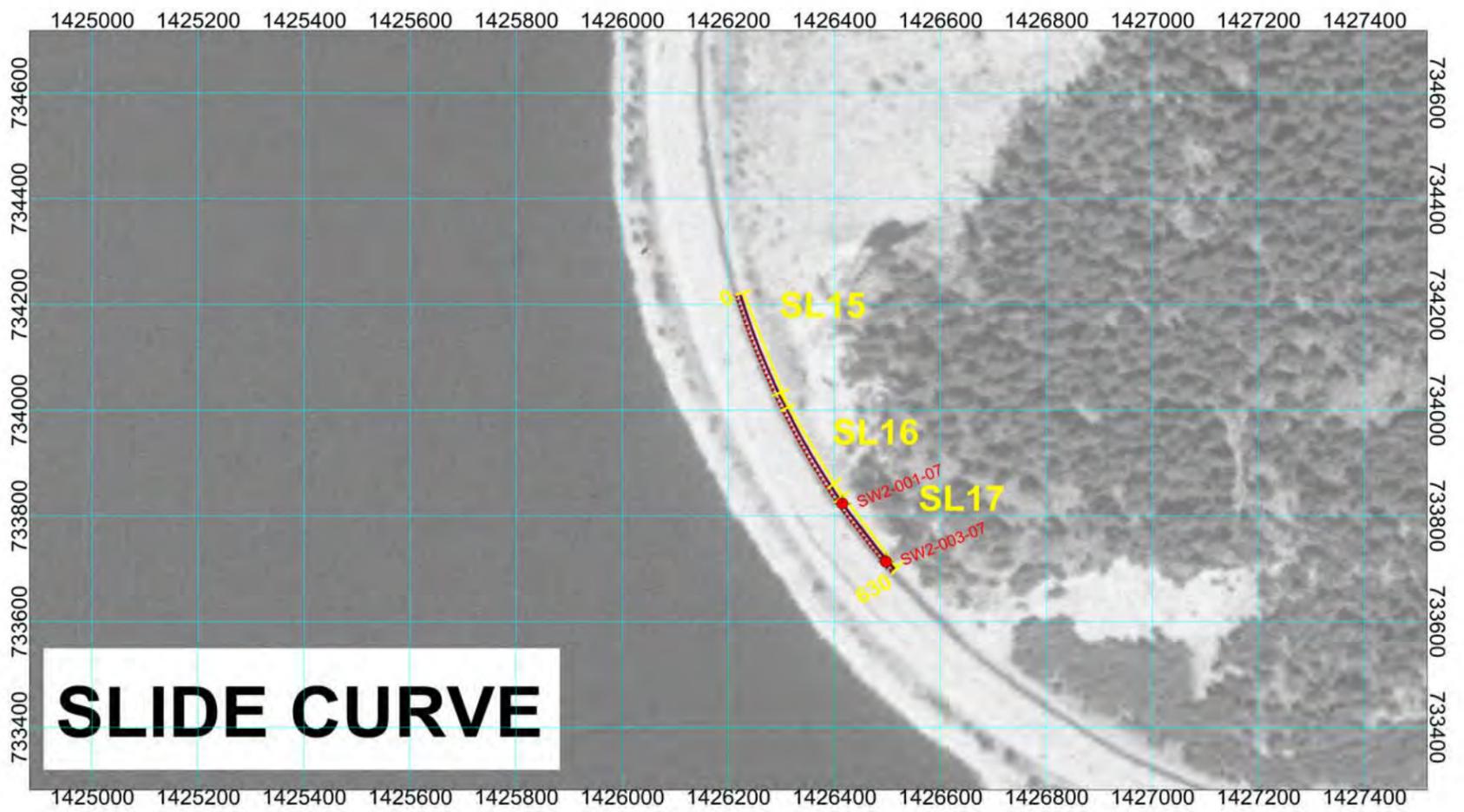
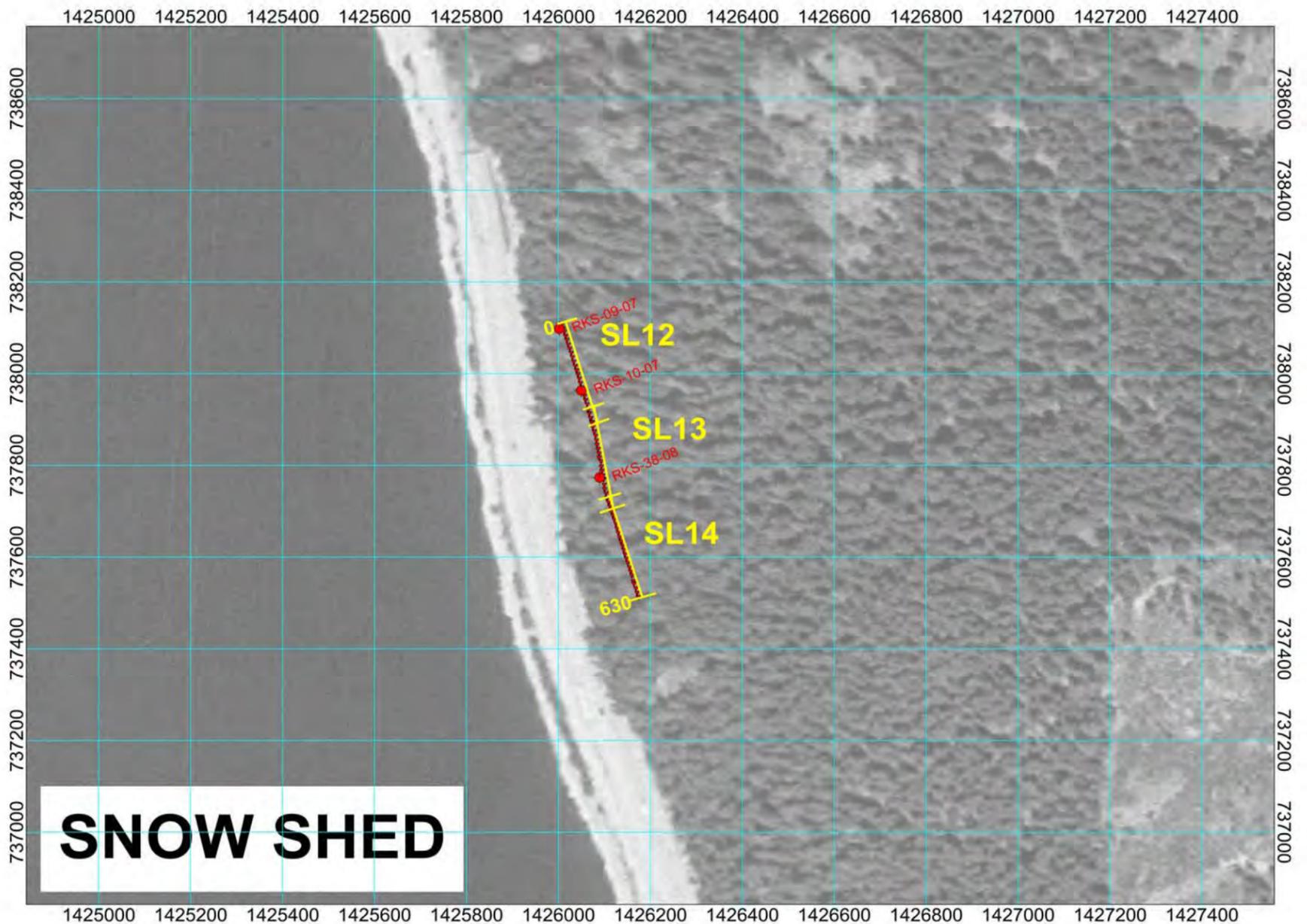
Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington

Prepared by:



Northwest  
Geophysical  
Associates, Inc.  
P.O. Box 1063, Corvallis, Oregon 97339





- LEGEND**
-  SEISMIC LINE
  -  REFRACTION GEOPHONE
  -  MASW GEOPHONE
  -  WSDOT BORING LOCATION

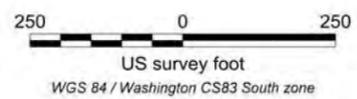
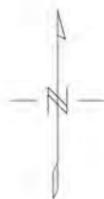


FIGURE 2

**PRELIMINARY**

Prepared by:



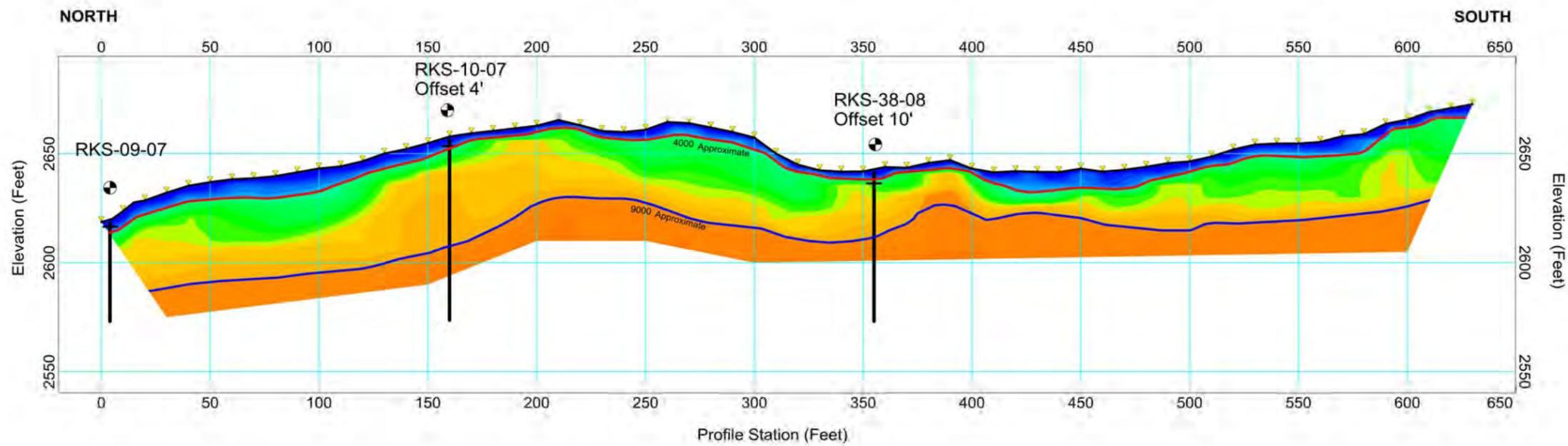
Northwest Geophysical Associates, Inc.

**Seismic Survey Plan**

Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington



### Seismic Lines 12-14

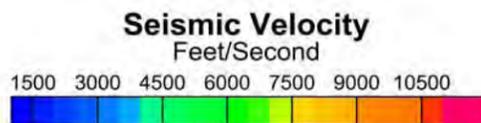


#### LEGEND

- GEOPHONE
- WSDOT BORINGS with OFFSET FROM PROFILE

#### INTERPRETATION

- 4000 (Approximate) WEATHERED BEDROCK - APPROXIMATELY 4000 ft/sec
- 9000 (Approximate) COMPETENT BEDROCK - APPROXIMATELY 9000 ft/sec



**PRELIMINARY**

FIGURE 3

#### Seismic Lines 12-14

Interpreted Velocity Section  
Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington

NGA PROJECT #: 717

FILENAME: SL12-14.map

DATE: OCT-2009

REVISION: A-28-OCT-09

Prepared By:

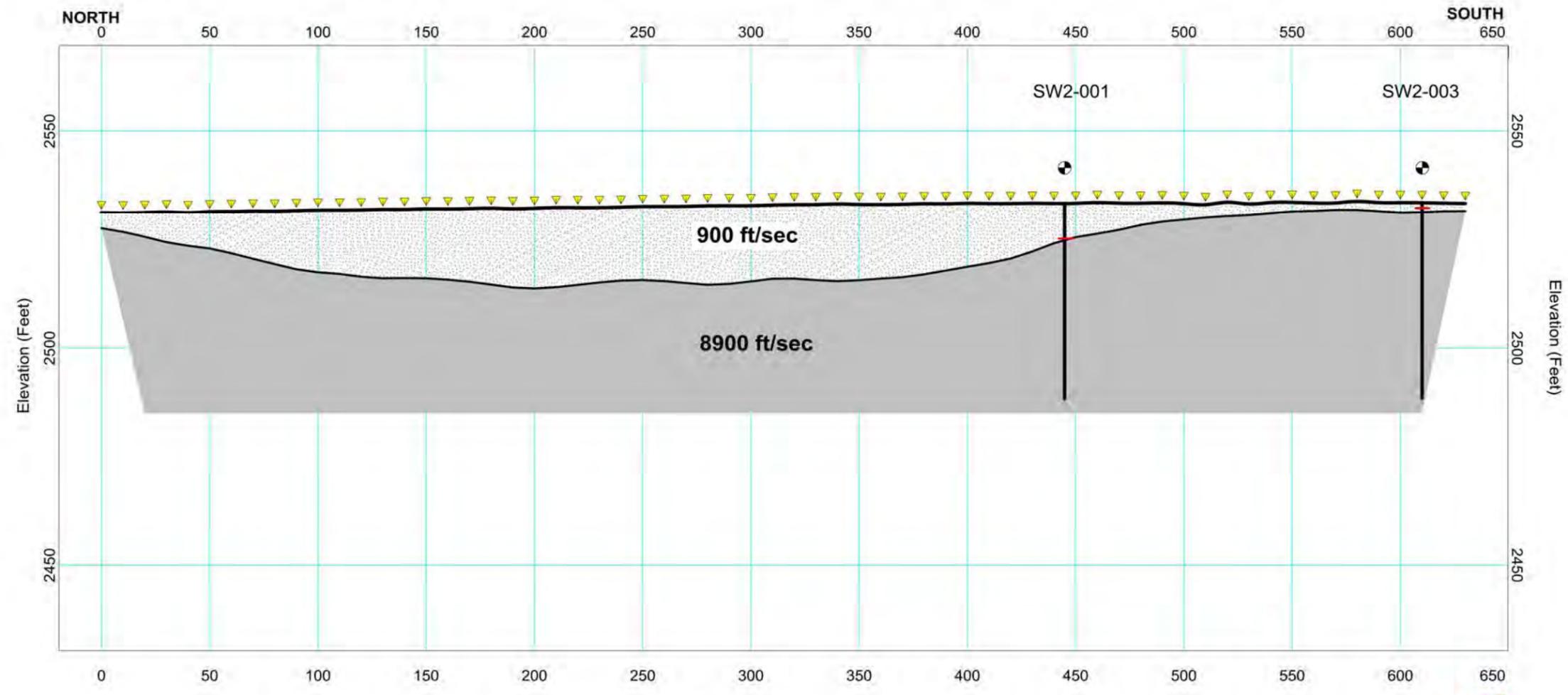


**Northwest  
Geophysical  
Associates, Inc.**



### Seismic Lines 15-17

APPROXIMATE WSDOT STATIONING: 1403+00, 1404+00, 1405+00, 1406+00, 1407+00, 1408+00, 1409+00



**PRELIMINARY**

#### LEGEND

- GEOPHONE
- WSDOT BORINGS

#### INTERPRETATION

- FILL/SLIDE DEBRIS  
(Gravel with some sand and silt - from WSDOT)  
- APPROXIMATELY 900 ft/sec
- COMPETENT ROCK  
(Lapilli Tuff / Andesite - from WSDOT)  
- APPROXIMATELY 8900 ft/sec

FIGURE 4

#### Seismic Lines 15-17

Interpreted Velocity Section  
Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington

NGA PROJECT #: 717  
 FILENAME: SL1-3SC.map  
 DATE: OCT-2009  
 REVISION: C-1-DEC-09



**Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington**

*Attachment A*

**Seismic Depth Interpretation Spreadsheet**

SL	gp	X-Loc	Surface SURF. ELEV.	Top of L2 elev	ROCK L3 elev	Top of L2 depth	ROCK L3 depth	NORTHING	EASTING
12	1	0	2618.6	2612.7	2582.4	NA	NA	738106.8	1426007.1
12	2	10	2623.6	2616.1	2584.7	NA	NA	738099.6	1426009.5
12	3	20	2628.5	2622.1	2586.8	NA	NA	738091.3	1426012.7
12	4	30	2632.1	2625.3	2588.5	NA	NA	738082.3	1426015.7
12	5	40	2635.2	2628.0	2590.1	NA	NA	738073.5	1426018.5
12	6	50	2637.0	2629.0	2591.2	NA	NA	738063.7	1426021.6
12	7	60	2638.2	2629.6	2591.8	NA	NA	738054.8	1426024.6
12	8	70	2638.7	2629.7	2592.6	NA	NA	738045.2	1426027.8
12	9	80	2639.7	2629.8	2593.4	NA	NA	738036.0	1426030.4
12	10	90	2641.5	2631.0	2594.5	NA	NA	738026.3	1426033.5
12	11	100	2643.2	2632.9	2595.6	NA	NA	738016.8	1426036.7
12	12	110	2644.2	2636.3	2596.4	NA	NA	738007.4	1426039.8
12	13	120	2646.5	2640.5	2597.2	NA	NA	737998.8	1426042.5
12	14	130	2650.0	2643.2	2600.1	NA	NA	737989.3	1426045.3
12	15	140	2652.1	2645.67	2602.68	NA	NA	737979.4	1426047.9
12	16	150	2654.8	2648.63	2604.68	NA	NA	737970.3	1426050.1
12	17	160	2657.9	2652.82	2607.86	NA	NA	737961.6	1426054.8
12	18	170	2659.3	2656.08	2610.36	NA	NA	737951.5	1426056.4
12	19	180	2660.4	2657.38	2615.34	NA	NA	737942.5	1426058.9
12	20	190	2661.6	2658.32	2620.86	NA	NA	737933.2	1426062.4
12	21	200	2662.8	2660.08	2626.68	NA	NA	737923.1	1426064.5
12	22	210	2665.4	2661.64	2629.75	NA	NA	737913.5	1426066.7
12	23	220	2663.1	2661.05	2630.16	NA	NA	737902.5	1426070.4
12	24	230	2660.8	2658.04	2629.83	NA	NA	737894.7	1426072.2
13	1	200	2663.1	2660.08	2626.68	NA	NA	737922.4	1426064.8
13	2	210	2664.2	2661.64	2629.75	NA	NA	737912.7	1426066.9
13	3	220	2663.0	2661.05	2630.16	NA	NA	737903.4	1426070.0
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13	5	240	2659.9	2656.82	2629.66	NA	NA	737883.7	1426074.9
13	6	250	2660.9	2656.6	2627.71	NA	NA	737874.2	1426078.1
13	7	260	2664.5	2658.21	2624.65	NA	NA	737864.6	1426080.8
13	8	270	2664.0	2658.23	2620.82	NA	NA	737854.7	1426083.0
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13	11	300	2657.3	2651.74	2616.19	NA	NA	737826.2	1426088.3
13	12	310	2650.0	2646.58	2613.6	NA	NA	737817.2	1426088.7
13	13	320	2644.9	2642.26	2611.22	NA	NA	737808.6	1426092.7
13	14	330	2642.3	2640.38	2609.8	NA	NA	737797.7	1426093.2

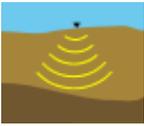
SL	gp	X-Loc	Surface SURF. ELEV.	Top of L2 elev	ROCK L3 elev	Top of L2 depth	ROCK L3 depth	NORTHING	EASTING
13	15	340	2641.8	2638.51	2609.63	NA	NA	737788.5	1426095.0
13	16	350	2641.8	2638.6	2610.87	NA	NA	737778.3	1426096.7
13	17	360	2643.8	2639	2613.9	NA	NA	737769.0	1426098.3
13	18	370	2643.5	2641.43	2617.87	NA	NA	737757.9	1426099.9
13	19	380	2645.7	2642.48	2625.02	NA	NA	737748.6	1426100.3
13	20	390	2647.0	2643.39	2626.4	NA	NA	737739.0	1426103.8
13	21	400	2643.1	2641.11	2622.69	NA	NA	737729.6	1426106.4
13	22	410	2641.9	2636.5	2620.43	NA	NA	737719.8	1426109.7
13	23	420	2641.9	2633.42	2621.84	NA	NA	737710.7	1426112.2
13	24	430	2641.6	2632.25	2622.77	NA	NA	737701.2	1426115.1
14	1	400	2642.7	2641.11	2622.69	NA	NA	737729.2	1426105.4
14	2	410	2641.4	2636.5	2620.43	NA	NA	737719.9	1426109.0
14	3	420	2642.0	2633.42	2621.84	NA	NA	737710.5	1426112.4
14	4	430	2641.6	2632.25	2622.77	NA	NA	737700.7	1426115.2
14	5	440	2641.6	2633.43	2622.23	NA	NA	737691.4	1426118.6
14	6	450	2643.1	2634.56	2620.62	NA	NA	737682.1	1426121.5
14	7	460	2641.7	2634.26	2618.29	NA	NA	737672.5	1426125.0
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14	9	480	2644.0	2636.84	2615.71	NA	NA	737653.2	1426130.5
14	10	490	2645.7	2639.33	2614.92	NA	NA	737643.7	1426133.2
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14	15	540	2654.6	2648.9	2619.1	NA	NA	737597.2	1426147.8
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14	17	560	2655.4	2648.9	2620.4	NA	NA	737577.8	1426154.7
14	18	570	2658.0	2649.6	2621.6	NA	NA	737568.5	1426157.5
14	19	580	2659.0	2651.1	2622.7	NA	NA	737559.7	1426159.9
14	20	590	2663.6	2658.3	2623.9	NA	NA	737547.8	1426163.2
14	21	600	2665.6	2662.5	2625.7	NA	NA	737542.6	1426164.2
14	22	610	2669.1	2665.21	2628.48	NA	NA	737532.1	1426168.6
14	23	620	2670.9	2666.39	2630.84	NA	NA	737522.4	1426171.1
14	24	630	2672.7	2666	2631.91	NA	NA	737512.9	1426173.9
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15	2	10	2531.1	2526.8	NA	4.32	NA	734205.9	1426218.3
15	3	20	2531.2	2525.7	NA	5.53	NA	734196.5	1426221.4

SL	gp	X-Loc	Surface SURF. ELEV.	Top of L2 elev	ROCK L3 elev	Top of L2 depth	ROCK L3 depth	NORTHING	EASTING
15	4	30	2531.3	2524.4	NA	6.9	NA	734187.0	1426224.7
15	5	40	2531.2	2523.6	NA	7.56	NA	734177.7	1426227.8
15	6	50	2531.4	2523.0	NA	8.4	NA	734168.2	1426231.0
15	7	60	2531.4	2521.9	NA	9.55	NA	734158.6	1426234.4
15	8	70	2531.5	2520.6	NA	10.92	NA	734149.2	1426237.7
15	9	80	2531.5	2519.4	NA	12.13	NA	734139.8	1426241.1
15	10	90	2531.6	2518.2	NA	13.45	NA	734130.6	1426244.6
15	11	100	2531.7	2517.5	NA	14.25	NA	734121.1	1426248.3
15	12	110	2531.8	2517.1	NA	14.66	NA	734111.8	1426251.8
15	13	120	2531.8	2516.5	NA	15.35	NA	734102.4	1426255.6
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15	19	180	2532.2	2514.7	NA	17.54	NA	734047.5	1426279.5
15	20	190	2532.1	2514.0	NA	18.11	NA	734038.4	1426283.5
16	1	200	2532.2	2513.8	NA	18.43	NA	734029.6	1426287.7
16	2	210	2532.4	2514.1	NA	18.29	NA	734020.5	1426292.0
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16	4	230	2532.3	2515.1	NA	17.22	NA	734002.8	1426301.0
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16	6	250	2532.6	2515.7	NA	16.92	NA	733984.8	1426310.2
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16	9	280	2532.8	2514.6	NA	18.15	NA	733958.2	1426323.7
16	10	290	2532.8	2514.8	NA	17.98	NA	733949.5	1426328.5
16	11	300	2532.8	2515.4	NA	17.41	NA	733940.8	1426333.3
16	12	310	2532.9	2516.0	NA	16.94	NA	733932.2	1426338.1
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16	16	350	2533.1	2515.7	NA	17.42	NA	733898.1	1426358.2
16	17	360	2533.1	2516.0	NA	17.02	NA	733889.1	1426363.8
16	18	370	2533.1	2516.3	NA	16.76	NA	733880.7	1426369.0
16	19	380	2533.2	2517.0	NA	16.24	NA	733872.2	1426374.3
16	20	390	2533.2	2517.9	NA	15.35	NA	733863.8	1426379.6
16	21	400	2533.3	2518.8	NA	14.59	NA	733855.3	1426385.2

**Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington**

*Attachment B  
NGA Technical Note*

**Seismic Refraction**



# Geophysical Services

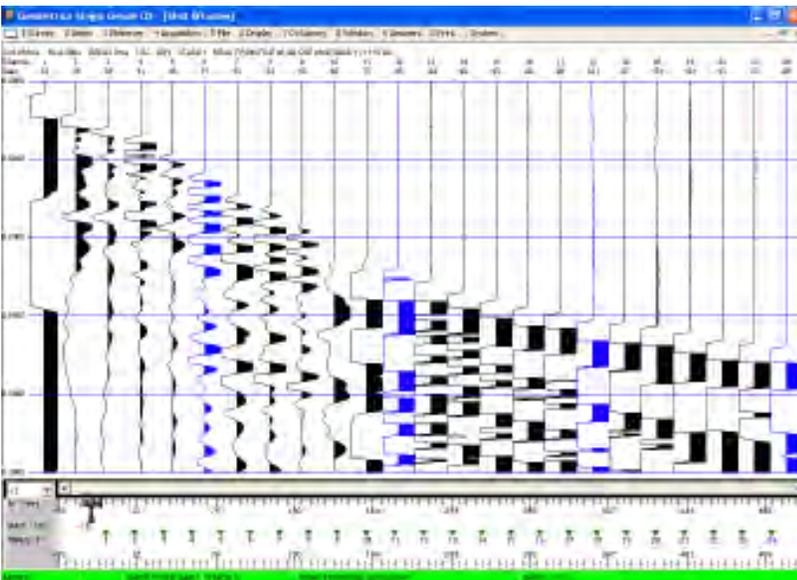
Environmental • Groundwater • Geotechnical

*TECHNICAL  
NOTE*



## Seismic Refraction

*Depth-to-Bedrock  
Competence of Bedrock  
Fault Mapping  
Groundwater Investigations*



COVER rev.2B, AUG. 2006

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## SEISMIC REFRACTION

### INTRODUCTION

Seismic refraction is a commonly used geophysical technique to determine depth-to-bedrock, competence-of-bedrock, depth-to-water table, or depth to other seismic velocity boundaries. Seismic refraction is widely used in geotechnical and groundwater investigations. It is one of the classic geophysical techniques, presented in most introductory geophysical courses and texts.

### PHYSICAL PRINCIPLES

The seismic refraction technique is illustrated in the schematic drawing, Figure 1. An impulsive source creates a seismic wave (sound wave), which travels through the earth. When source to receiver separation is small, the first arrival is generally the direct wave propagating through the surface layer. When the wave-front reaches a layer of higher velocity (e.g. bedrock),

a portion of the energy is refracted, or bent, and travels along the refractor as a head wave, at the seismic velocity of the refractor (bedrock). Energy from the propagating head wave leaves the refractor at the critical angle of refraction and returns to the surface, where its arrival is detected by a series of geophones and recorded on a seismograph. The angle of refraction depends on the ratio of velocities in the two materials (Snell's Law). Travel times for the impulsive wave-front to reach each geophone are measured from the seismograph records. From those travel times, seismic velocities in each layer, and depths to each layer can be calculated.

Seismic refraction differs from seismic reflection, which is widely used in petroleum exploration. In reflection seismic the waves reflected off the geologic interface are utilized rather than the refracted arrivals.

### FIELD PROCEDURES

When acquiring seismic refraction data a series of 12 or 24 geophones (receivers) are placed along the seismic line at set intervals, the *geophone interval*. Each of these set-ups of 12 or more geophones is termed a *spread*. The geophone interval is generally 5 to 50 feet depending on the desired resolution and the desired depth of exploration. Due to the geometry of refraction (governed by Snell's Law), it is necessary for the length of

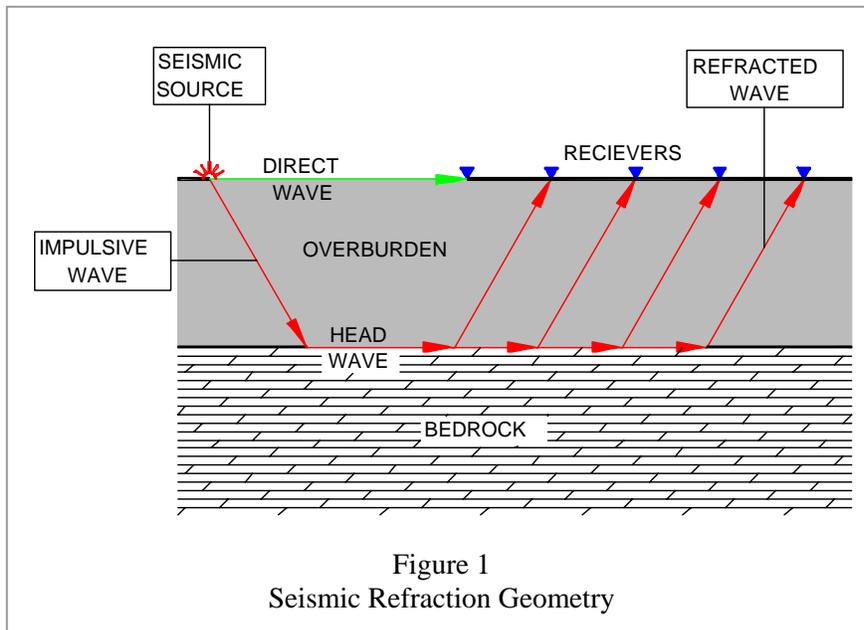


Figure 1  
Seismic Refraction Geometry

the seismic spread to be approximately 3 to 5 times the depth of the overburden in order to detect the primary refractor (i.e., the bedrock).

A series of 5 to 11 *shots* are initiated for each spread, one at each end, one or more beyond the ends (off end), and one or more along the spread. These additional shotpoints allow dipping interfaces, changes in overburden materials, and intermediate layers to be identified and resolved, increasing the accuracy of the depth-to-bedrock interpretation. Several spreads may be put together to form a longer refraction profile line.



Figure 2  
Airless Jack Hammer

Several options are available for the seismic energy source. A sledge hammer striking an metal plate is one of the simplest and most common sources. An airless jackhammer (Figure 2) provides additional energy and may be effective if the bedrock is 30 or 40 feet, particularly if the overburden is sufficiently consolidated. A higher energy source, such as an elastic weight drop (Figure 3) or 8-gauge seismic shells, may be required if the overburden is



Figure 3  
Elastic Weight Drop

loose and poorly consolidated, or if the bedrock interface is significantly deeper. Small explosive charges may be used where depth of exploration, near surface attenuation, or high ambient noise demand a stronger source.

## DATA PROCESSING & INTERPRETATION

Processing begins with picking the time of the first arrival. While several automated picking programs are available, the first arrival times must be checked for accuracy and consistency and adjusted as needed.

Two basic methods of processing first arrival data are available, each with its strengths and limitations. These can be classified as delay-time techniques and tomographic techniques. Figures 4 and 5 show delay-time and tomographic interpretations of the same data.

## Delay-Time Techniques

Delay-time methods treat the earth as discrete layers, with refracted waves coming off each layer. Generally, these methods assume laterally continuous, constant velocity representations of the subsurface. Delay-time methods are sometimes referred to as the plus-minus method or the time-term method. The generalized reciprocal method (GRM) is based on the delay-time approach. Historically, the delay-time is the classic method of interpreting seismic refraction data. Redpath, 1973, is an

excellent reference for the basic delay-time technique.

Picked first arrivals are assigned a specific subsurface layer from which that seismic wave was refracted. This is a critical step in the interpretation process. Layer assignment is based on the slope of the travel-time curves and the recognition of parallel curve segments from adjacent shots.

Reduced traveltimes are calculated using refracted arrivals from each direction, effectively removing changing layer thickness on the velocity curve and leading to a better velocity

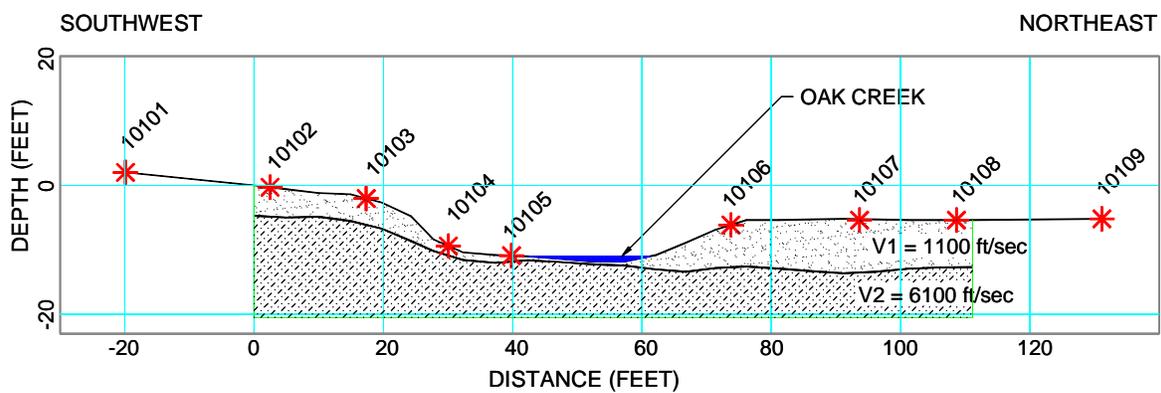


Figure 4  
Layered Earth Interpretation

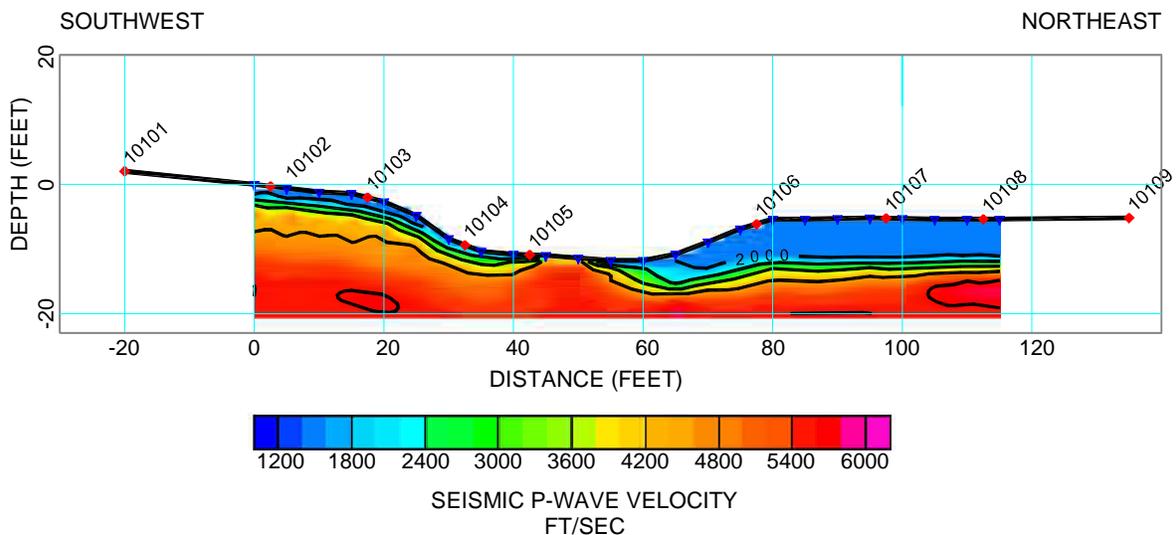


Figure 5  
Tomographic Interpretation

determination. Refractor depth is computed from the velocity and delay time, at each geophone.

A common enhancement to the delay time method is to refine the model, using ray tracing. The ray tracing procedure adjusts the depth of the interface at the point the ray enters or emerges from the refractor.

### **Tomographic Techniques**

Tomographic techniques partition the earth model into a mesh of finite-element or finite-difference *elements*, each with an assigned velocity. Element velocities are iteratively adjusted, altering the theoretical ray path(s) through the model. Velocities are adjusted until a best fit between the observed travel-times, and the modeled travel-times is arrived at.

Tomographic methods evolved rapidly in the late 1990s (Zhu et al., 2000). Today there are several commercial software packages available. Each has its own variation of the inversion method, choice of the starting model, and introduction of geologic constraints (see Sheehan et al., 2005).

Tomographic solutions allow for both lateral and vertical changes in velocity. Thus, they can approximate complex geologic situations such as faulting and weathering. However, where sharp geologic/velocity boundaries are present, tomographic interpretations will tend to represent them as gradational boundaries.

Tomographic solutions are inherently non-unique. Several earth models may yield similar travel-time curves. Therefore, it is often useful to look at the layered earth/delay-time solution along with the tomographic solution and geologic constraints to select the most appropriate.

### **APPLICATIONS**

The product or deliverable from a seismic refraction survey is generally a profile, or cross section, along the seismic line showing depth-to-

bedrock (or other primary refractor) at each geophone, and seismic velocities in the bedrock and the overburden. Often layers with intermediate velocities (corresponding to layers or units with varying consolidation or lithology) can be identified and resolved.

Seismic velocities relate to the soundness or competence of rock, and to the degree of consolidation, cementation, and/or saturation in soils. The Caterpillar Tractor Co. has developed a series of tables which empirically relate seismic velocities to the *rippability* of bedrock with their equipment (such as a D8 or D9 with one or several ripper teeth). Figure 6 is an example of those rippability tables.

In geotechnical engineering, depth-to-bedrock and rippability surveys are commonly used for design and cost estimates for road cuts, pipelines, and other civil engineering projects. Groundwater applications of seismic refraction include mapping bedrock channels, identifying faults and fracture zones, and delineation of geologic boundaries to constrain hydrogeologic models.

### **LIMITATIONS**

There are several inherent limitations to the seismic refraction technique. Fortunately, for most projects, the required geologic conditions are met, and the seismic refraction program can be successfully. However, these conditions must always be considered when planning a refraction survey, and when examining refraction data.

- **Velocity contrast**

A velocity contrast between strata is required to refract the seismic wave. Hence, similar stratigraphic units are difficult to differentiate. Weathering zones with gradational changes present problems. Tomographic techniques can now handle some of the weathering zone problems

- **Velocity reversals**  
Standard refraction velocity analysis assumes that the velocity of successive layers increases with depth.
- **Hidden layers**  
Thin layers with small velocity contrast between adjacent layers may be hidden layers, and no first arrival refracted from that layer will be present. These layers are difficult to detect without additional borehole data.
- **Lateral velocity variations**  
Most standard interpretation software does not do a good job at handling geology with gross lateral variations in lithology. Fault zones can be detected from slower velocities

in the brecciated fault gouge zones, but often are poorly imaged unless a detailed survey with short geophone spacing is carried out.

- **Shot points coverage**  
Tomographic models require dense shot coverage to properly constrain the models. This is an operational or cost constraint rather than a constraint inherent in the method.
- **Urban Noise**  
Urban noise (traffic, railways, construction, etc.) and natural noise (wind, waves, moving water, etc.) often limit the depth of exploration, or dictate a more energetic source.

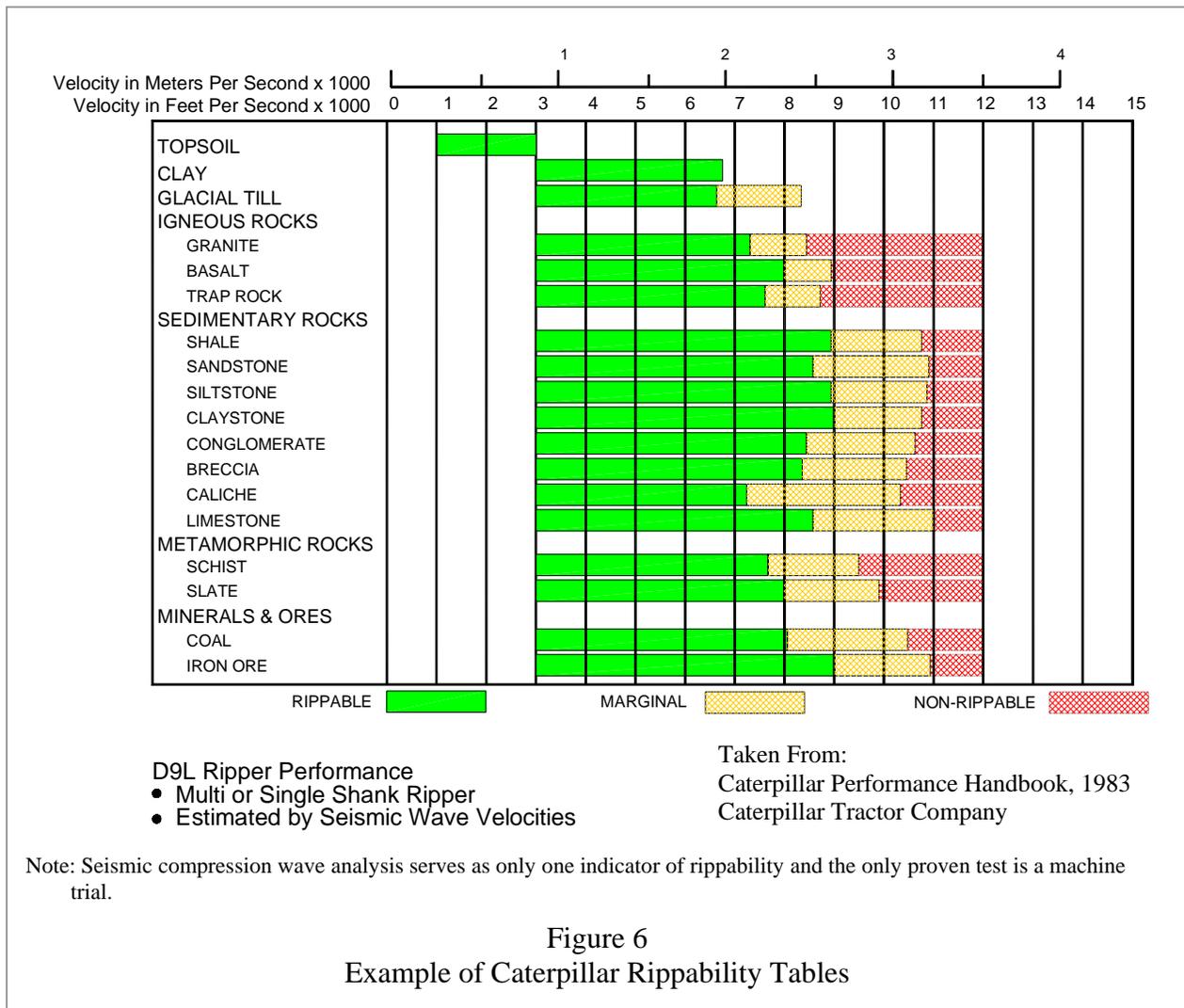


Figure 6  
Example of Caterpillar Rippability Tables

Borehole or ground truth data is very useful in constraining the seismic interpretation. Seismic data is often used very effectively to interpolate between borings or to extrapolate away from borings. Downhole seismic velocities surveys are even more effective at constraining refraction interpretations.

### **FURTHER READING:**

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Sheehan, J. R., Doll, W. E. and Mandell, W. A., 2005, An Evaluation of Methods and Available Software of Seismic Refraction Tomography Analysis, Journal of Environmental and Engineering Geophysics, v.10(1):21-34.

Zhu, T., Cheadle, S., Petrella, A., and Gray, S., 2000, First-arrival tomography: method and application: Expanded Abstracts SEG 70<sup>th</sup> Annual Meeting, 2028-2031p.

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## *DISCUSSION OF GEOPHYSICAL TECHNIQUES*

### **SEISMIC REFRACTION**

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refraction\_teq\_2006-08A.doc

SL	gp	X-Loc	Surface SURF. ELEV.	Top of L2 elev	ROCK L3 elev	Top of L2 depth	ROCK L3 depth	NORTHING	EASTING
17	1	410	2533.3	2519.6	NA	13.67	NA	733846.8	1426390.6
17	2	420	2533.3	2520.7	NA	12.63	NA	733838.7	1426395.9
17	3	430	2533.4	2522.3	NA	11.07	NA	733830.6	1426401.6
17	4	440	2533.3	2524.2	NA	9.13	NA	733822.5	1426407.3
17	5	450	2533.3	2525.6	NA	7.77	NA	733814.5	1426413.1
17	6	460	2533.5	2526.4	NA	7.1	NA	733806.4	1426418.9
17	7	470	2533.4	2527.3	NA	6.11	NA	733798.5	1426425.0
17	8	480	2533.4	2528.4	NA	4.96	NA	733790.3	1426431.4
17	9	490	2533.5	2529.2	NA	4.28	NA	733782.5	1426437.5
17	10	500	2533.3	2529.6	NA	3.63	NA	733774.8	1426443.6
17	11	510	2533.0	2530.1	NA	2.91	NA	733766.7	1426449.8
17	12	520	2533.6	2530.4	NA	3.15	NA	733758.8	1426456.0
17	13	530	2533.2	2530.7	NA	2.52	NA	733750.8	1426462.1
17	14	540	2533.5	2531.1	NA	2.44	NA	733742.9	1426468.2
17	15	550	2533.6	2531.4	NA	2.12	NA	733735.2	1426474.3
17	16	560	2533.4	2531.6	NA	1.84	NA	733727.5	1426480.6
17	17	570	2533.4	2531.8	NA	1.64	NA	733719.9	1426487.0
17	18	580	2533.8	2531.8	NA	1.98	NA	733712.0	1426493.4
17	19	590	2533.5	2531.5	NA	2.01	NA	733704.5	1426500.0
17	20	600	2533.5	2531.2	NA	2.29	NA	733697.0	1426506.4

#N/A	RKS-09-07	#N/A	2621.1	#N/A	2618.1	#N/A	3.0	738098.0	1426001.9
#N/A	RKS-10-07	#N/A	2527.0	#N/A	2523.8	#N/A	3.2	737963.5	1426049.6
#N/A	RKS-38-08	#N/A	2523.1	#N/A	2517.1	#N/A	6.0	737773.3	1426089.4

Rock depths for borings are based on WSDOT boring logs.

**Phase 1C Seismic Refraction Survey  
I-90 Snoqualmie Pass East Project  
Hyak, Washington**

*Attachment C  
NGA Technical Note*

**MASW**



## PHYSICAL PRINCIPLES

Surface wave methods examine the dispersion curve (velocity as a function of frequency) of Rayleigh surface waves to examine the near-surface, shear-wave velocity structure. Surface wave motion extends approximately  $\frac{1}{2}$  wavelength into the earth, the amplitude decaying exponentially with depth. Hence, higher frequencies (shorter wavelengths) are affected by the shallow materials, and the longer wavelengths are affected by deeper soils and/or rock. Thus, by measuring the changes in phase velocity with frequency we can determine the velocity structure with depth. The inversion process involves creating a hypothetical layered earth velocity model, calculating the theoretical surface wave response, and iteratively adjusting the velocities of the layers to minimize error between the calculated and observed response.

For Surface Wave analysis, NGA employs a combination of Multi-channel Analysis Surface Wave (MASW) and Micro-tremor Array Measurements (MAM). For the higher frequency, and shallow depth of investigation, the MASW uses an active impulsive source (e.g., a hammer) and a linear array of geophones. With the impulsive source, the surface wave train can be selected from the seismic records, omitting the earlier body waves (P-waves) from the analysis.

For the lower frequencies, and greater depth of investigation, MAM uses passive noise (e.g., traffic noise) and a 2 dimensional, L-shaped array. The L-shaped array allows determination of the direction of incidence of the signal onto the array. The MASW and MAM dispersion curves are then combined and inverted to arrive at the velocity model.

## FIELD PROCEDURES

NGA records seismic data on a Geometrics Geode 24 channel signal enhancement seismograph. Digital computer files of the waveforms are recorded during the field surveys. The digital computer records are subsequently utilized for detailed velocity analysis using SeisImager Wave Equation by Geometrics.

MASW data are recorded using a linear array of 24 4.5-Hz geophones. Generally the initial geophone interval is set at 5 feet. Seismic signals are initiated at 15 to 30 feet off both ends of the seismic line. NGA often uses an airless jackhammer to generate the seismic signal and repeated signals are stacked to enhance the signal. Data are recorded at 0.500 millisecond intervals for 2 seconds. The geophone interval is then increased to 10 feet and a seismic signal are initiated at 20 to 50 feet off both ends of the seismic line. Data are again recorded at 0.500 millisecond intervals for 2 seconds.

Following acquisition of MASW data, MAM data are collected. A two dimensional L-shaped array is laid out utilizing 11 4.5-Hz geophones spaced 30 feet apart. The corner of the L-array is centered on the point of interest. The two dimensional aspect of the method utilized by the MAM method is important in that it allows an azimuth to be determined on the passive surface wave. Twenty records of passive, ambient signal are recorded for 30 seconds at a 2 millisecond sample intervals.

## DATA PROCESSING AND INTERPRETATION

NGA uses SeisImager Wave Equation software by Geometrics of San Jose, California, to process

surface wave data. In this processing suite, a dispersion curve of the velocity of the surface waves as a function of frequency is generated for each MASW shot point and for the combined records of the MAM data. Nominally, twenty (20) MAM records are used in development of the passive dispersion curve. The dispersion curves for the MASW and MAM data are then merged and smoothed. A least squares inversion using forward modeling is then used to approximate earth structure.

## 2D MASW PROFILES

The MASW technique can be extended to obtain a series of dispersion curves by moving the array laterally and repeating the data acquisition process. The resulting one-dimensional models can then be put together in a profile to produce a 2D shear-wave velocity profile.

In practice this is done, laying out a linear array of geophones and “shooting” into the array, i.e. hammering along the array between every 1 or 2 geophones. Traces which have a common midpoint (CMP) between the source and receiver are then collected (gathered) into one record. Each trace in the gather will have a different distance between source and receiver. The dispersion curve is then evaluated for each gather and the resulting velocity model plotted at the midpoint.

## LIMITATIONS OF SURFACE WAVE ANALYSIS

Limitations of the seismic surface wave method include:

- The assumption of plane layers from source to receiver.
- Lateral inhomogeneities can produce errors not detectable in the data.
- Higher modes of dispersion which can give erroneous results.

## REFERENCES

- Martin, A.J. and Diehl, J.G. (2004), Practical experience using a simplified procedure to measure average shear-wave velocity to a depth of 30 m ( $V_{s30}$ ), 13th World Conference on Earthquake Engineering, Vancouver, B.C., Canada.
- Park, C.B., Miller, R.D. and Xia, J. (1999), Multichannel analysis of surface waves, *Geophysics*, 64(3): 800-803.
- Xia, J., Miller, R.D. and Park, C.B. (1999), Estimation of near-surface shear-wave velocity by inversion of Rayleigh waves, *Geophysics*, 64(3): 691-700.

FILE: MASW\_2007-07-21.doc  
REVISION: 21-Jul-07

**APPENDIX C.3**  
**Snowshed COBL® Logs**





Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 4, 2009

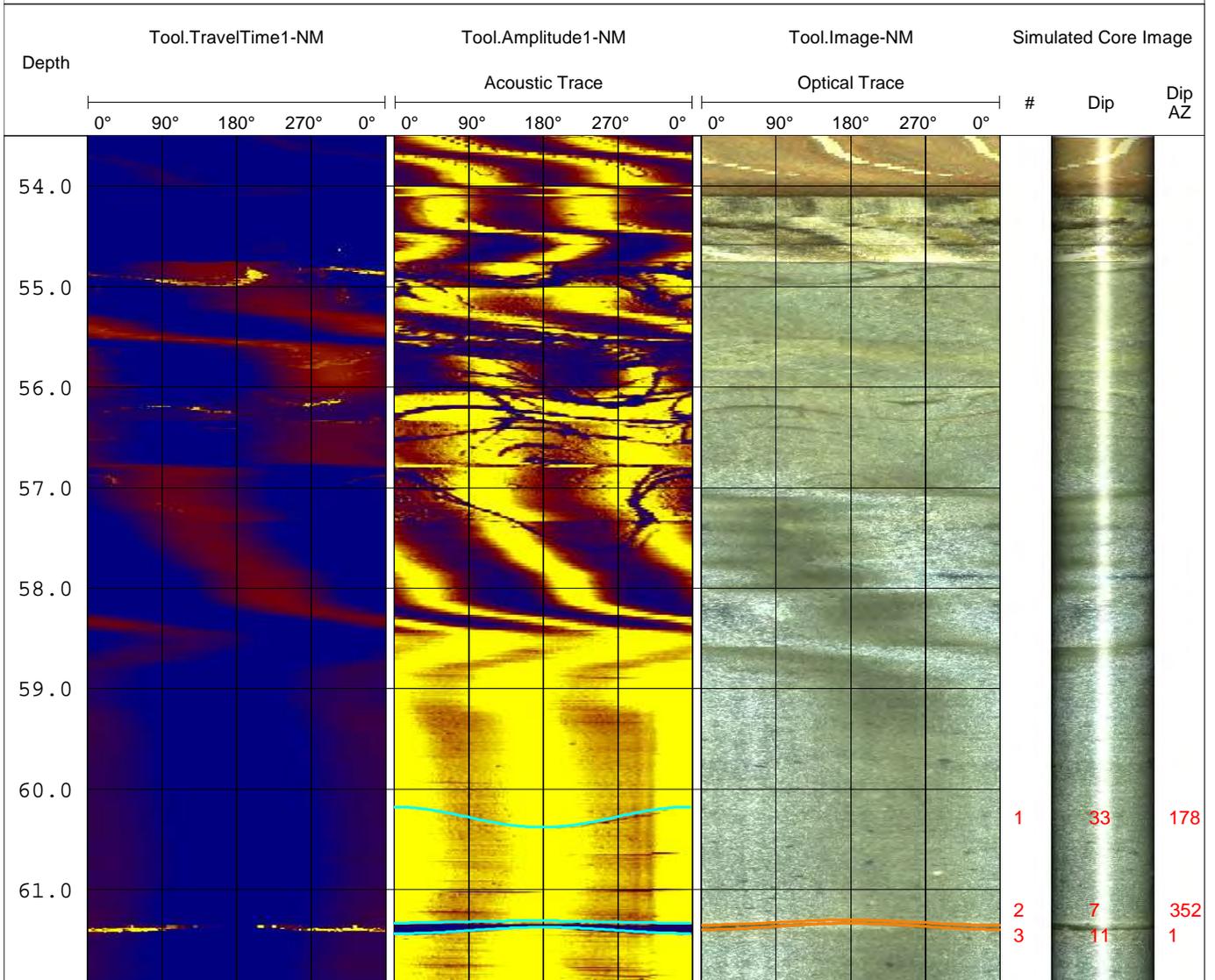
Borehole ID: SSD-006-09

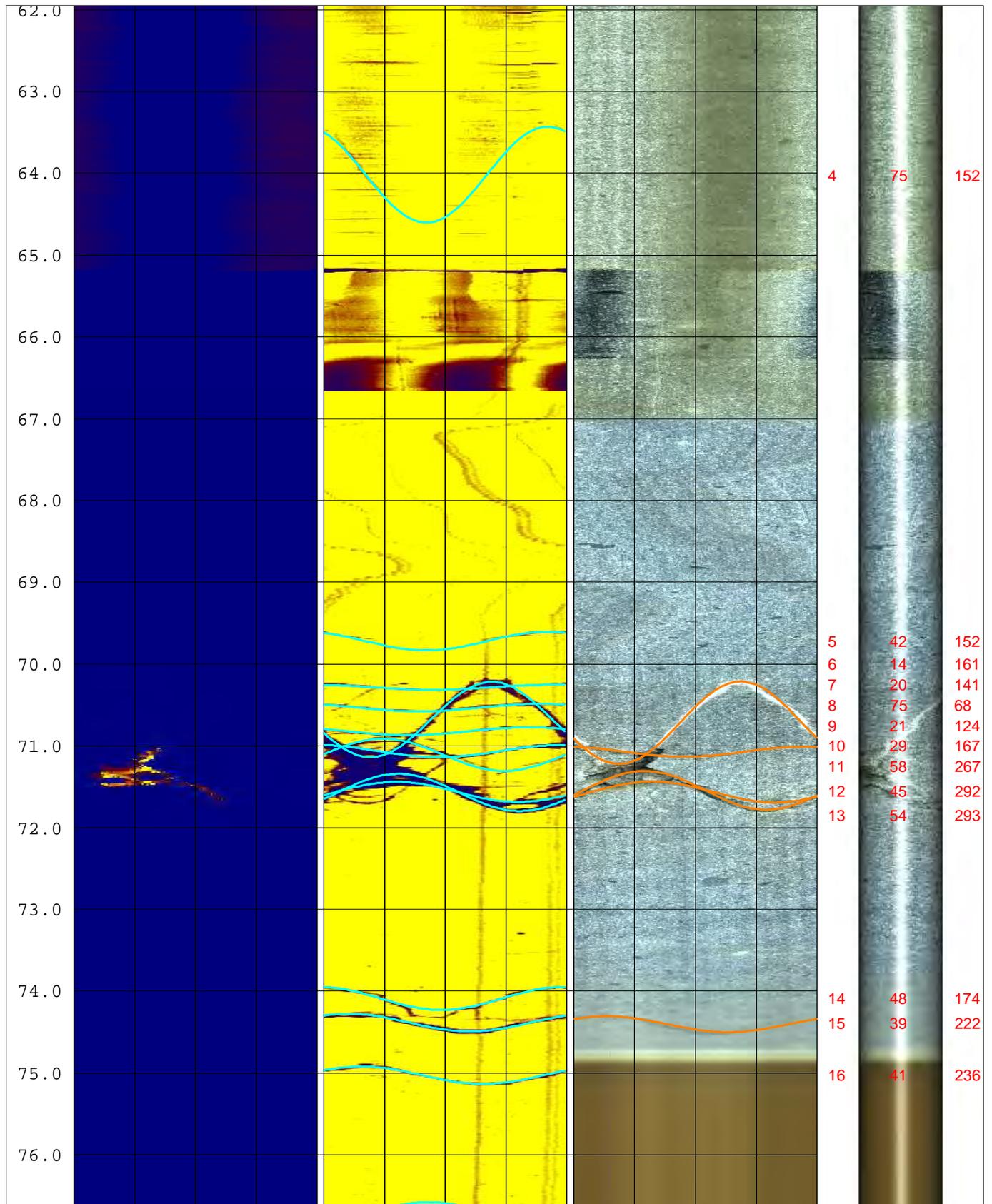
File Name: SSD00609-2M.wcl

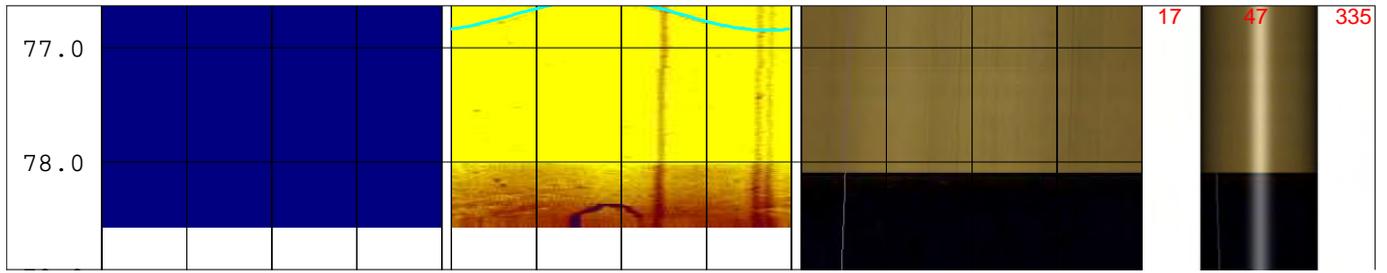
Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on HQ Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.
- 6) HQ hole 54.0-65.2, NQ hole 65.2-78.9.









Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 1, 2009

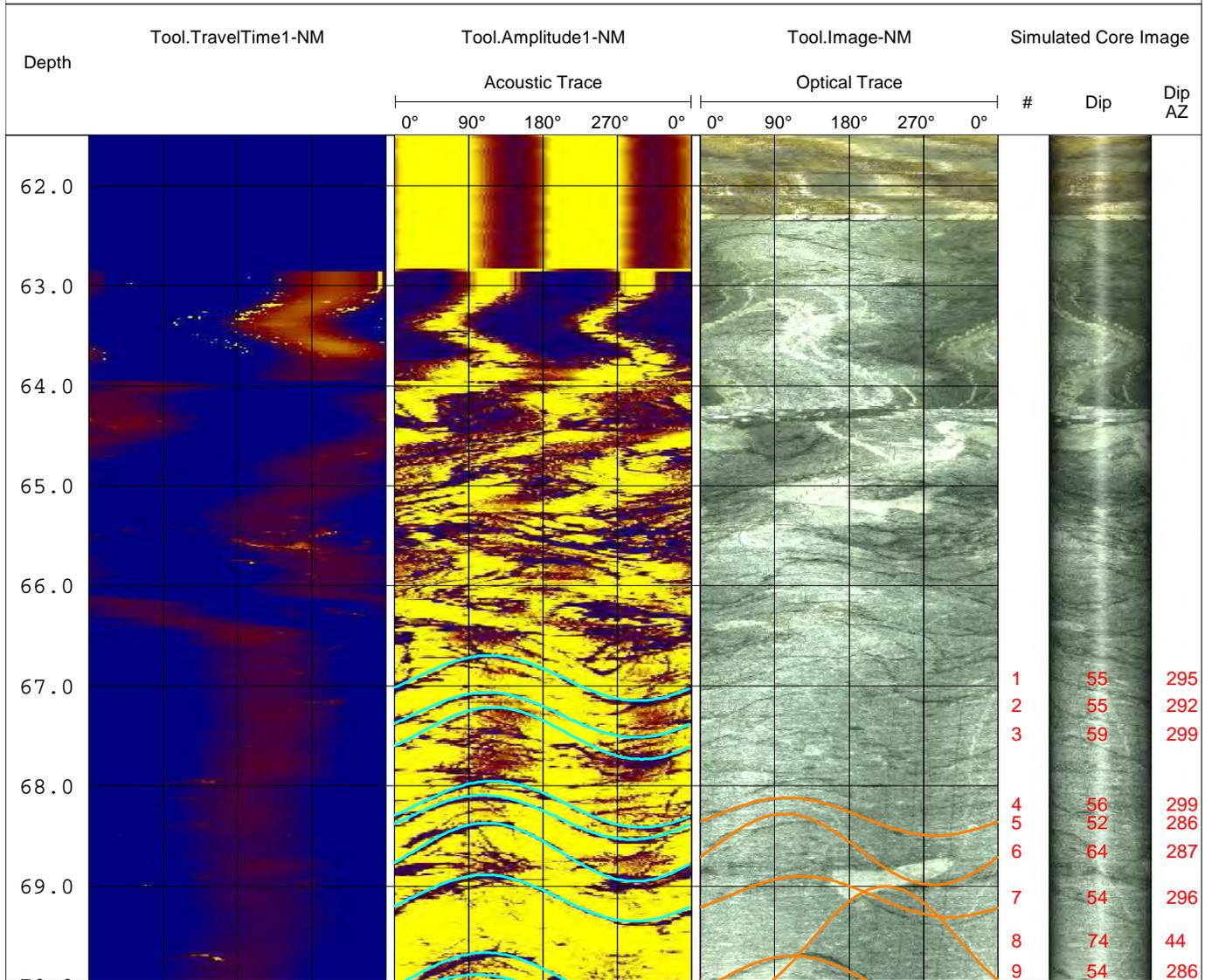
Borehole ID: SSD-007-09

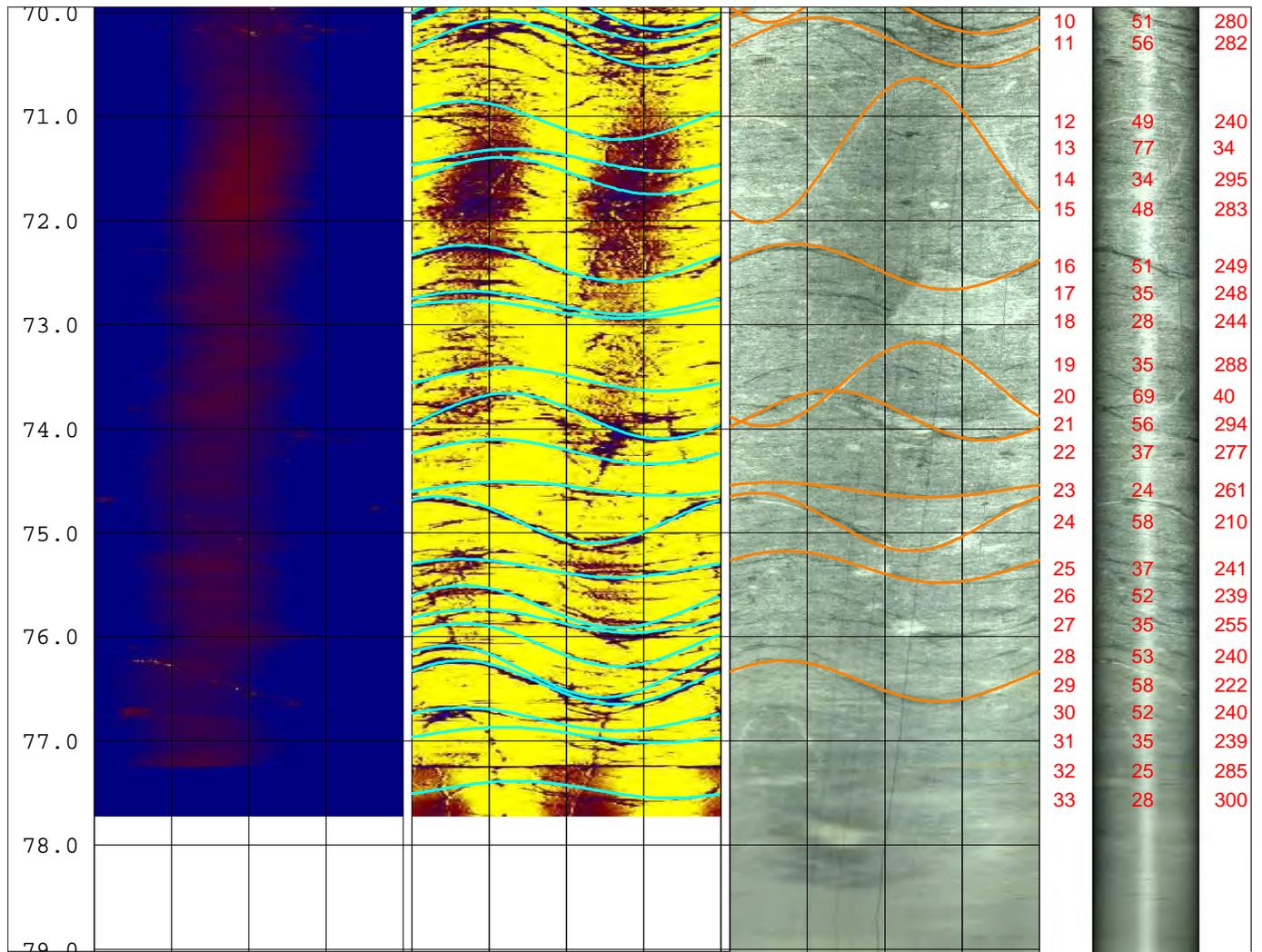
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Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.







Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 11, 2009

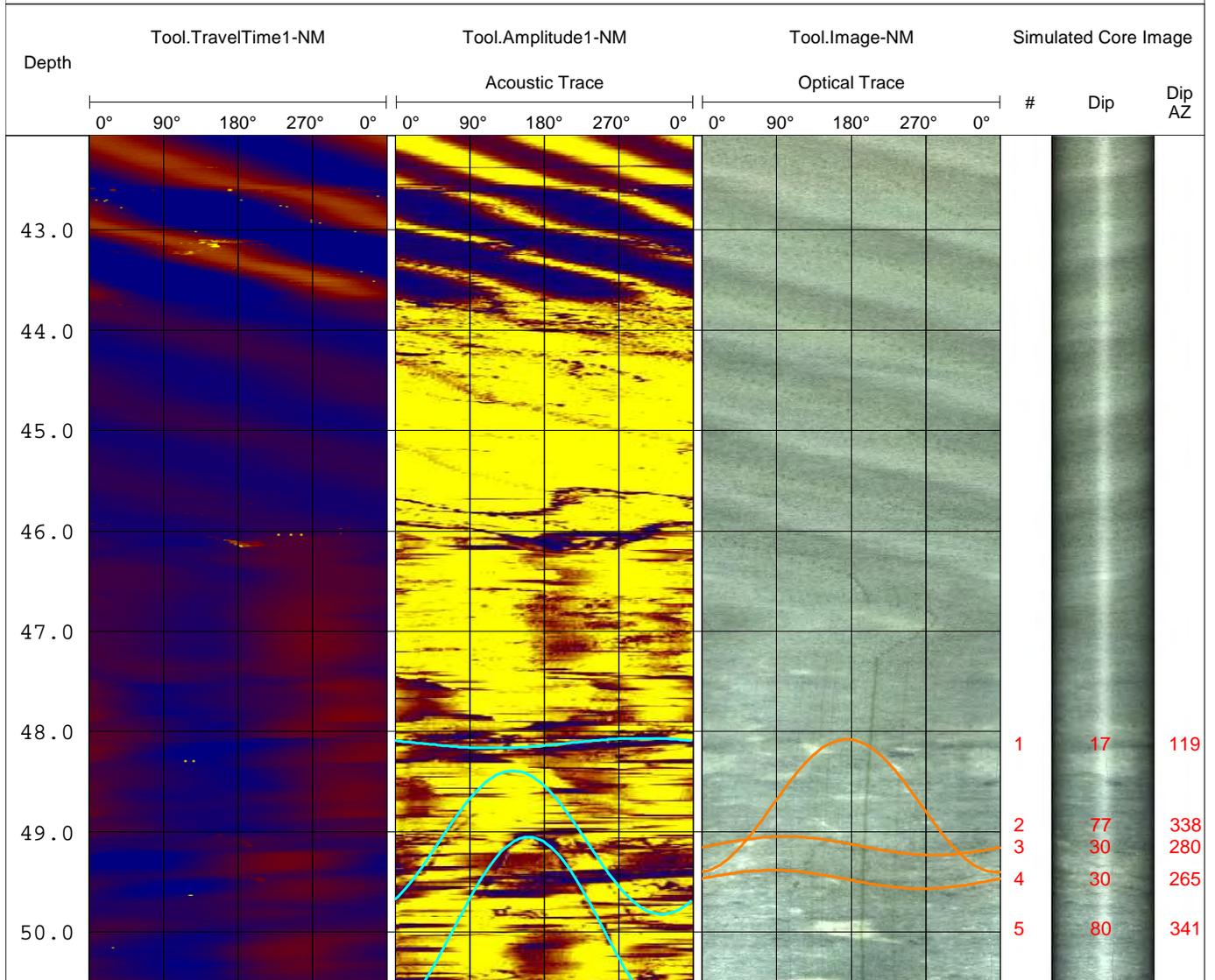
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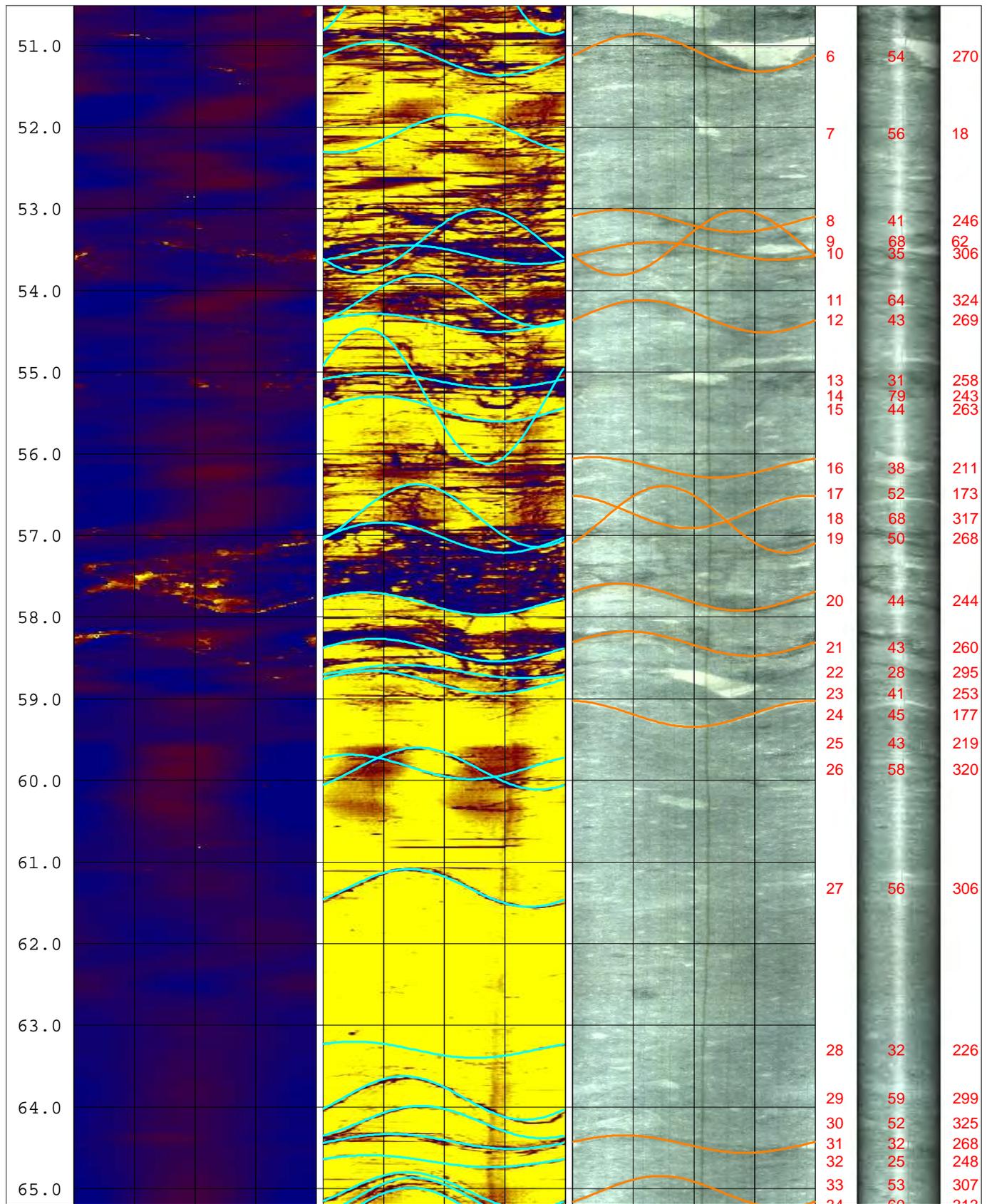
File Name: SSD00809-2M.wcl

Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.









Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 13, 2009

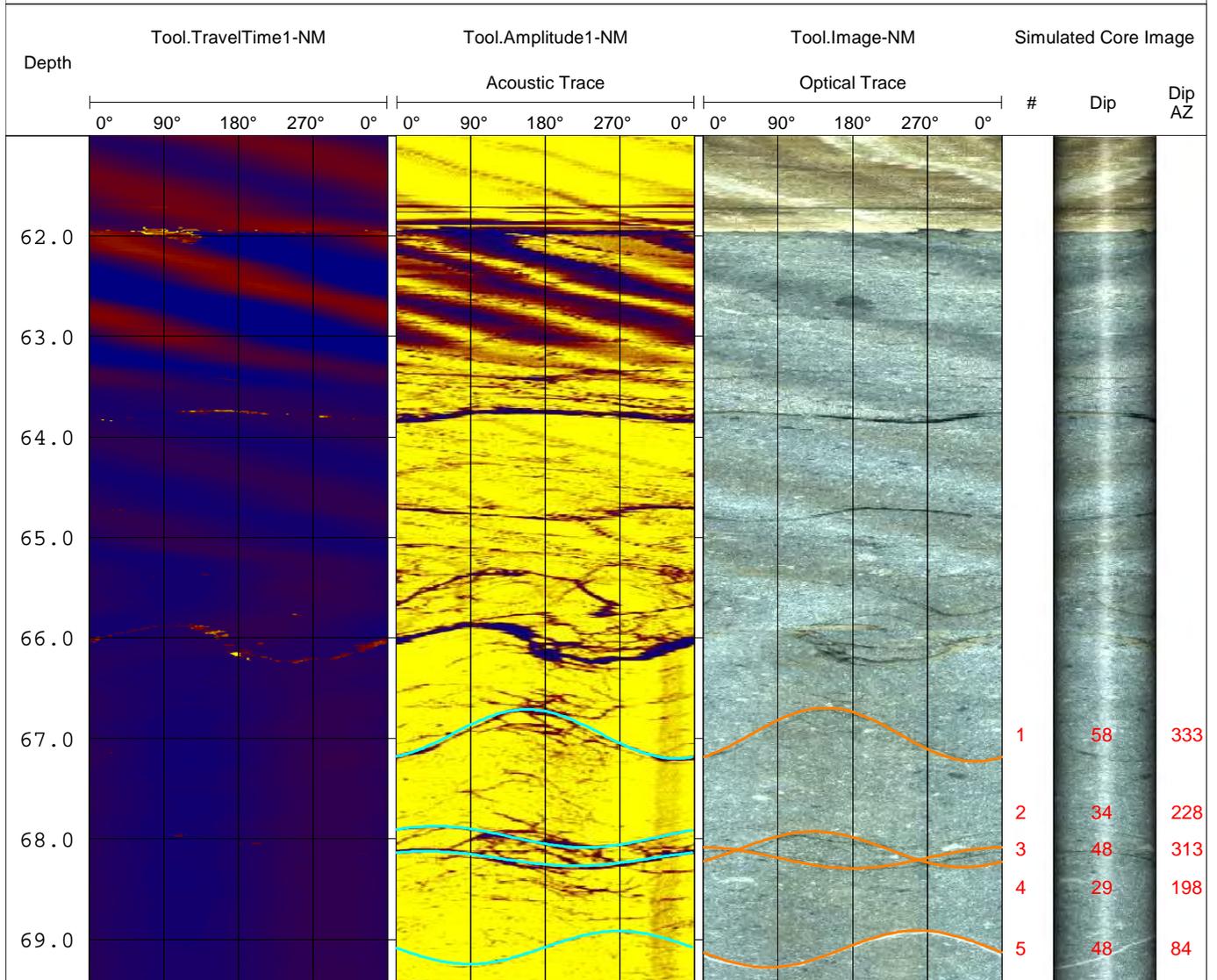
Borehole ID: SSD-009-09

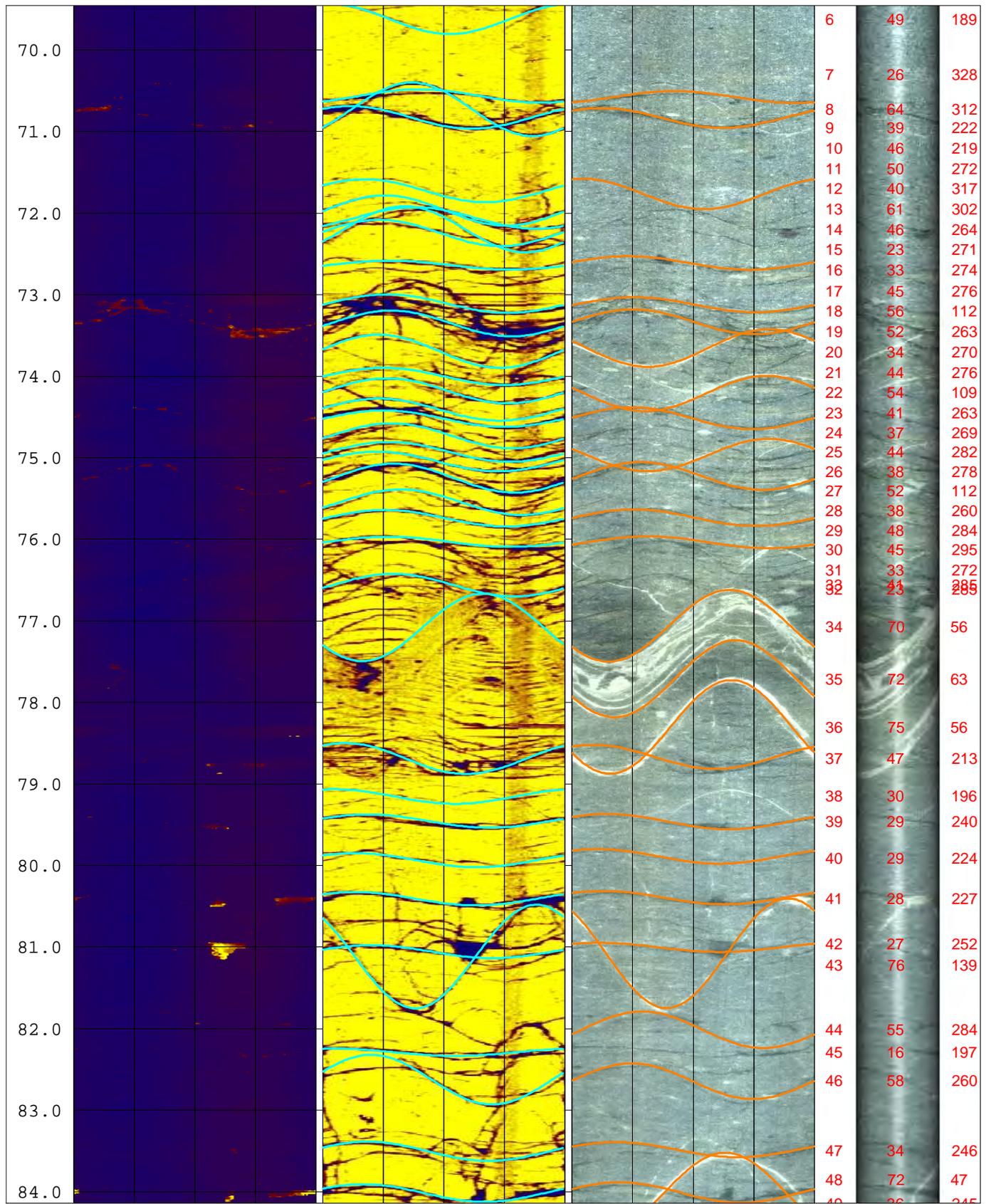
File Name: SSD00909-2M.wcl

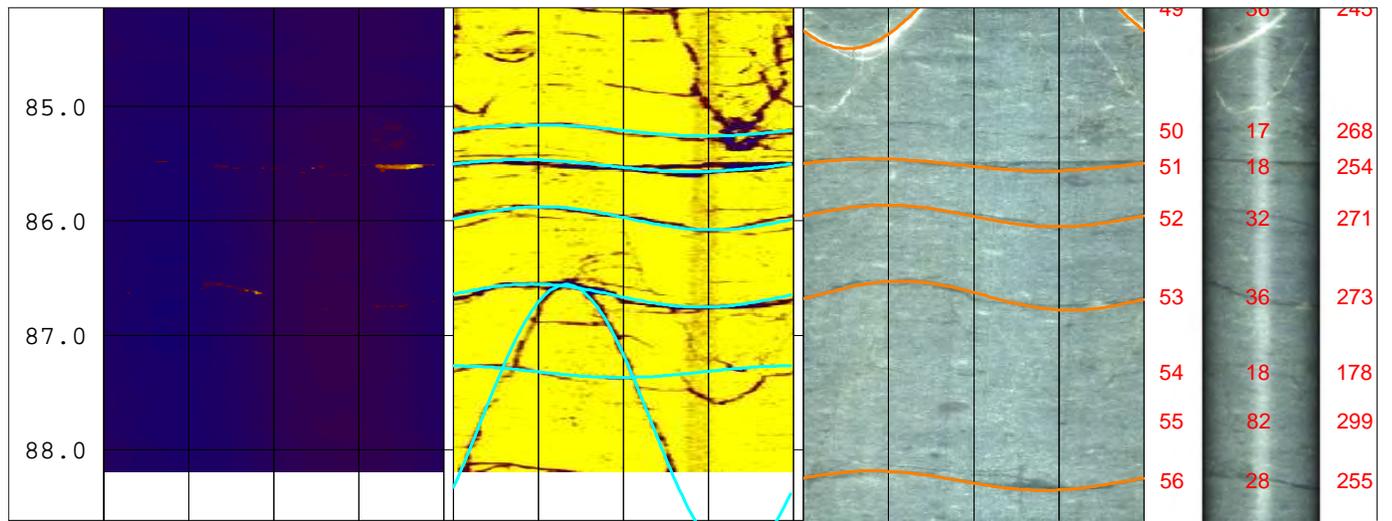
Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.









Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project:

Date Logged: June 16, 2009

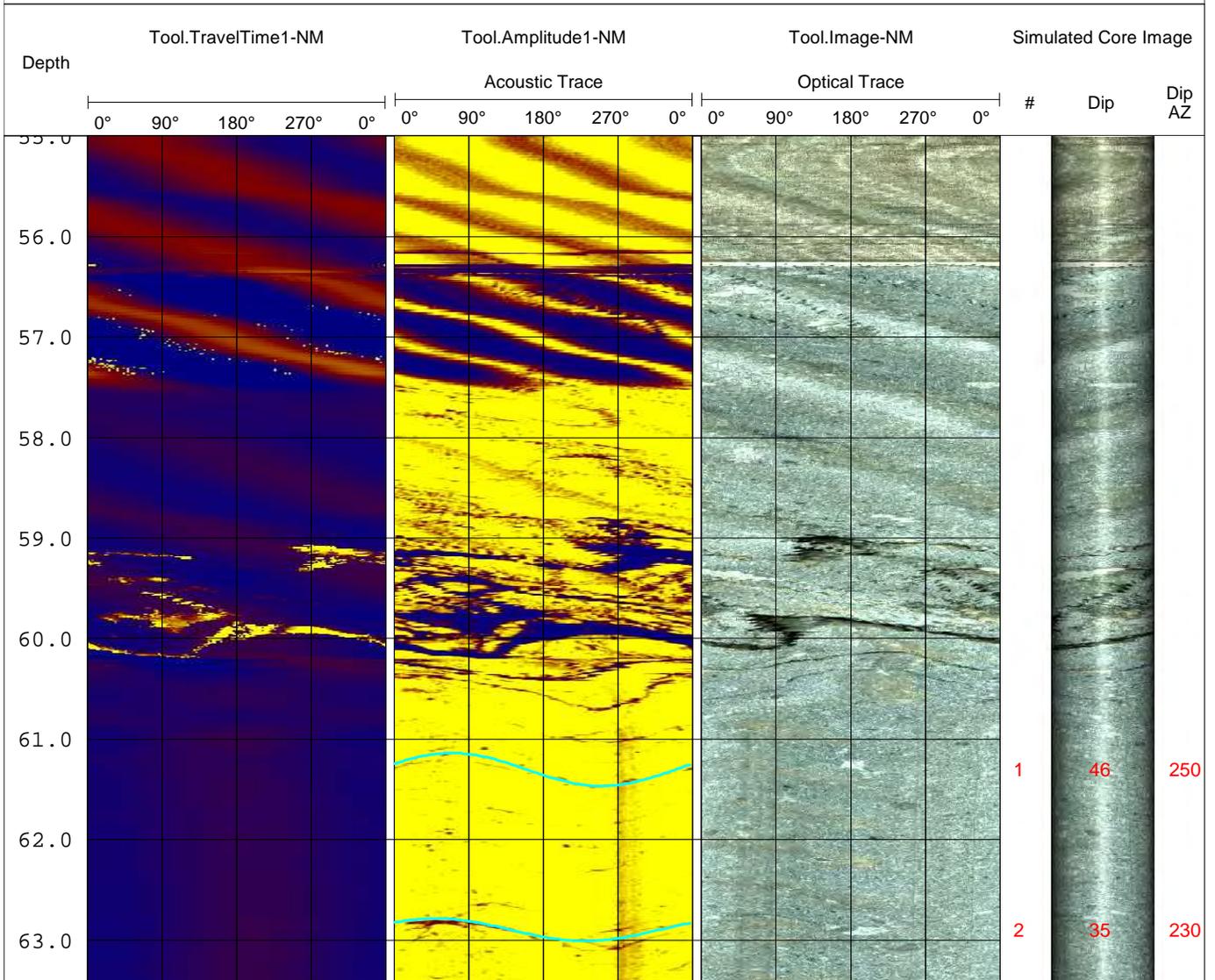
Borehole ID: SSD-010-09

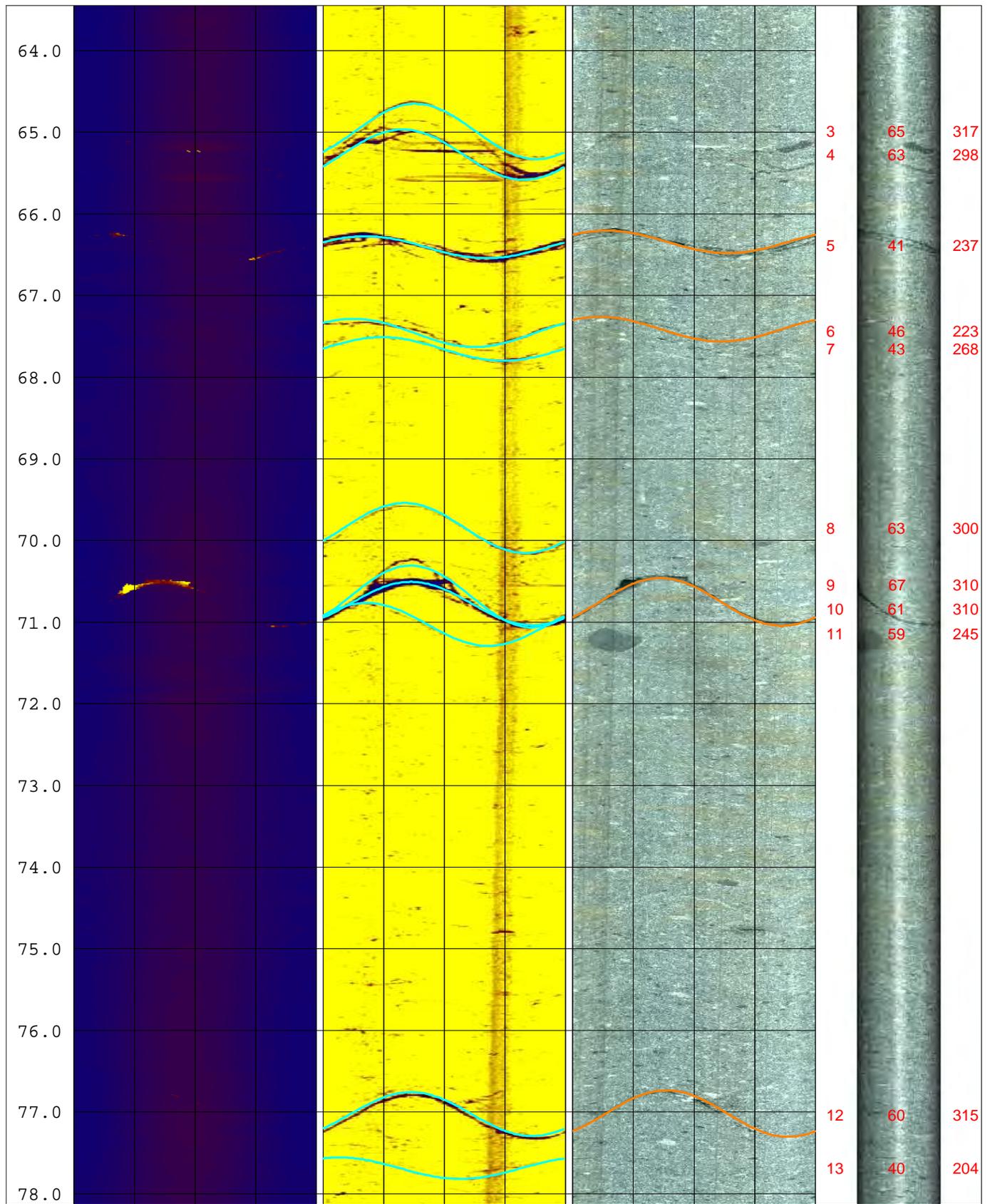
File Name: SSD01009-1M.wcl

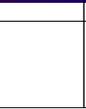
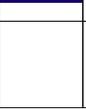
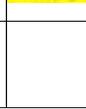
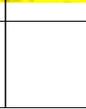
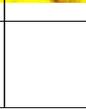
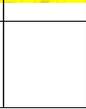
Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.





79.0													
	80.0												
													



Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 30, 2009

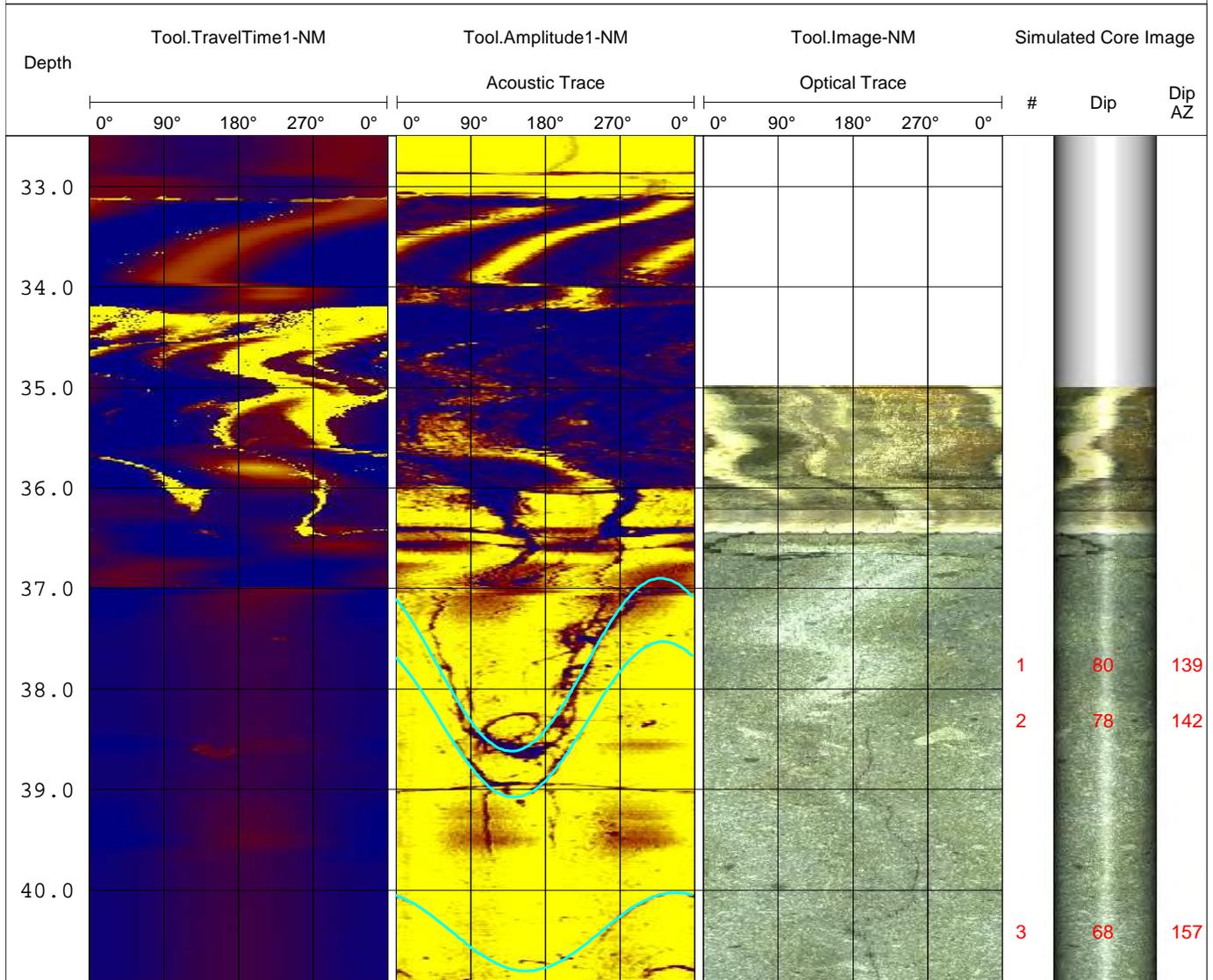
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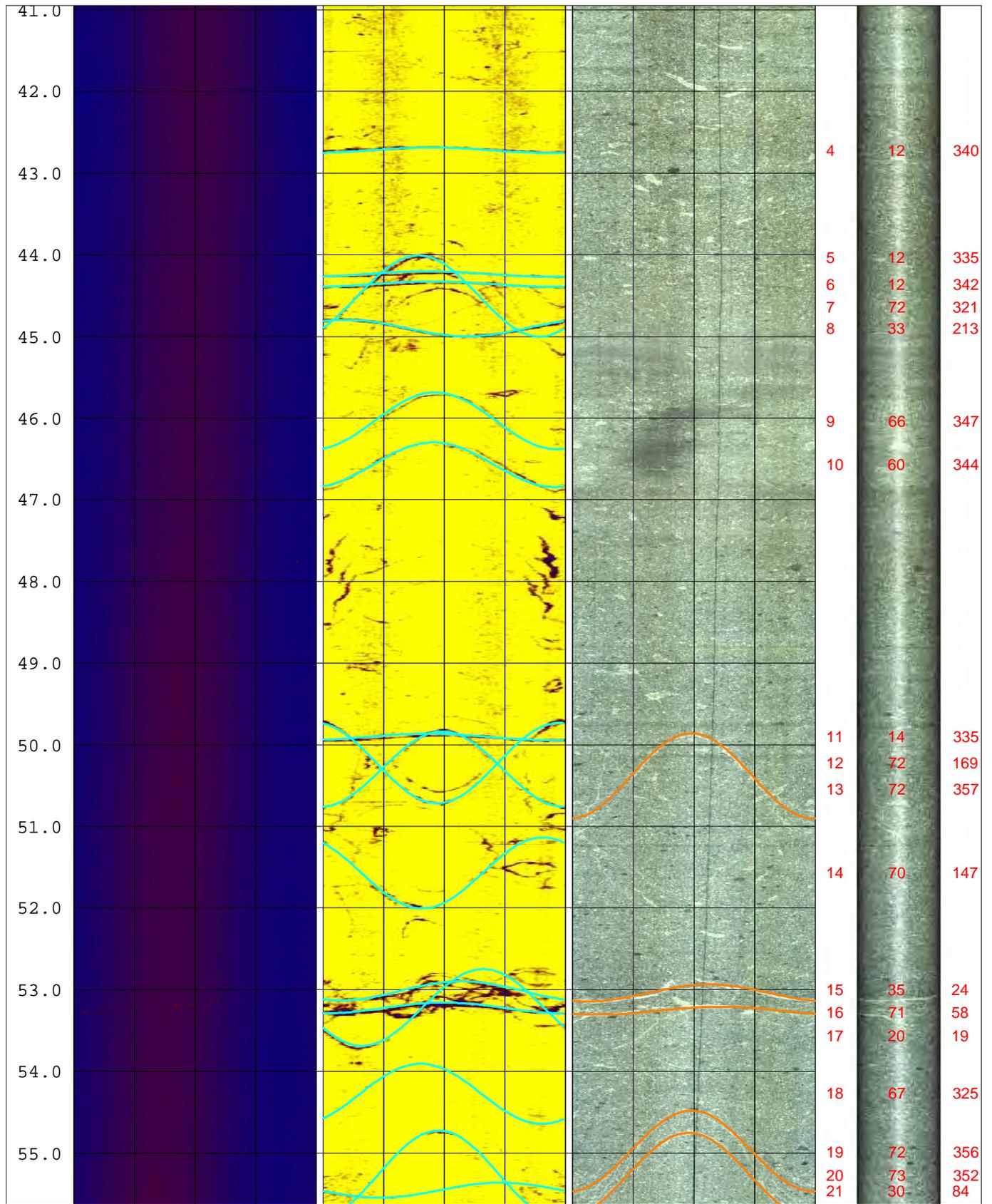
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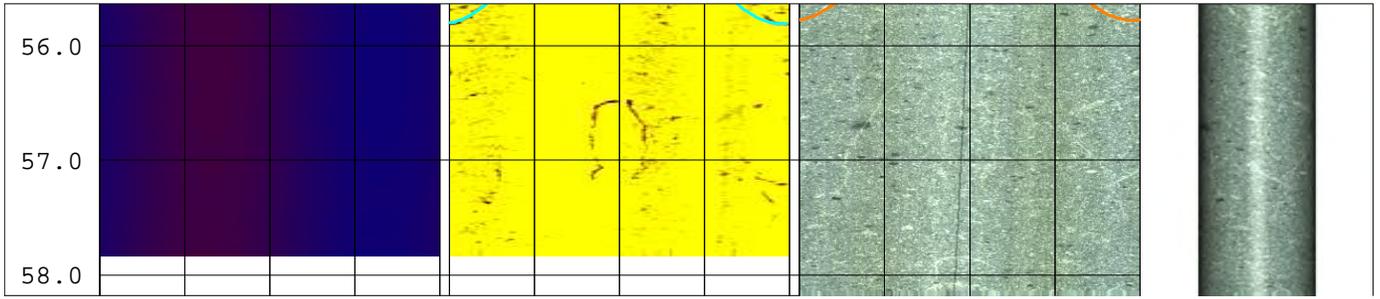
Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiever Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.









Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 24, 2009

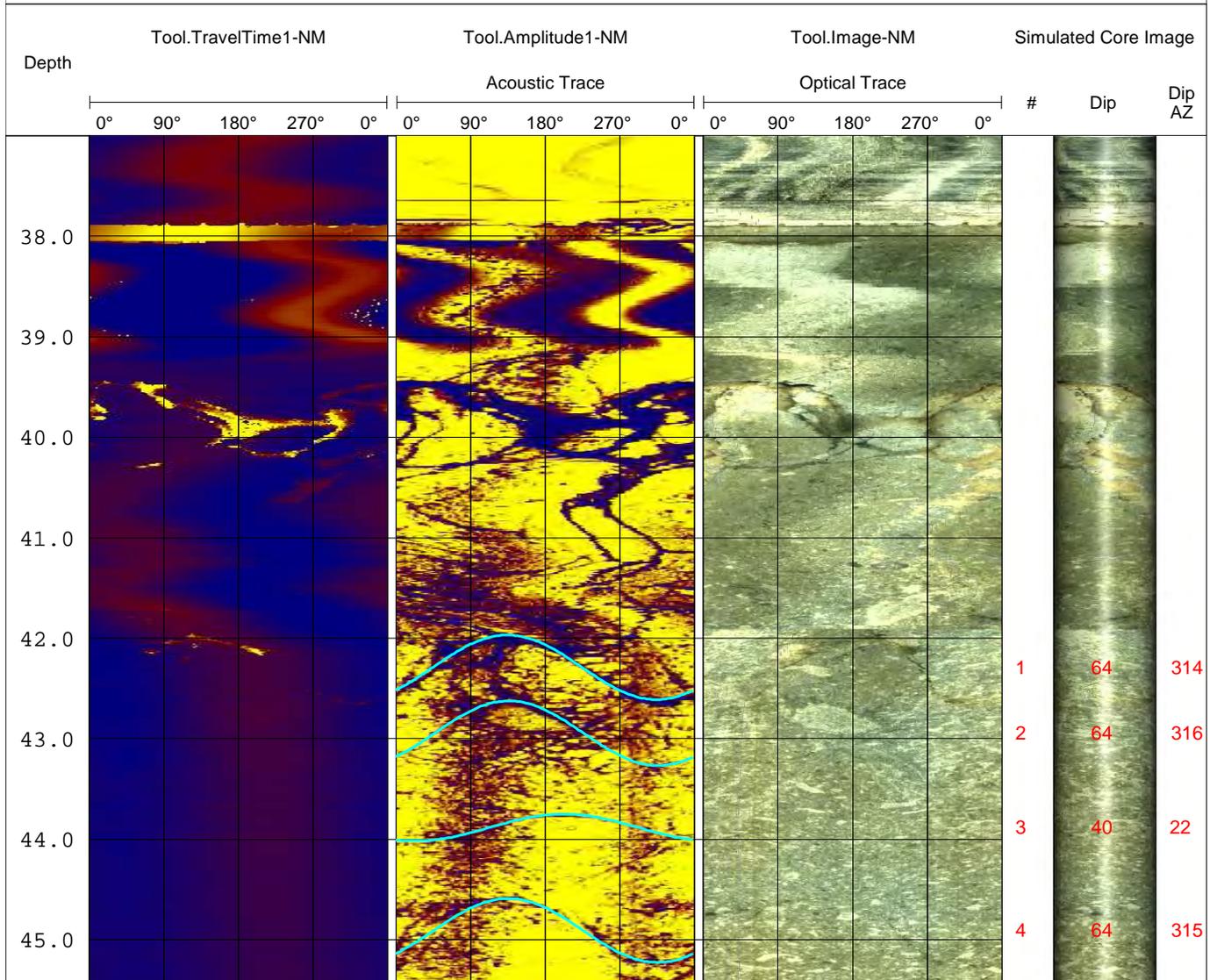
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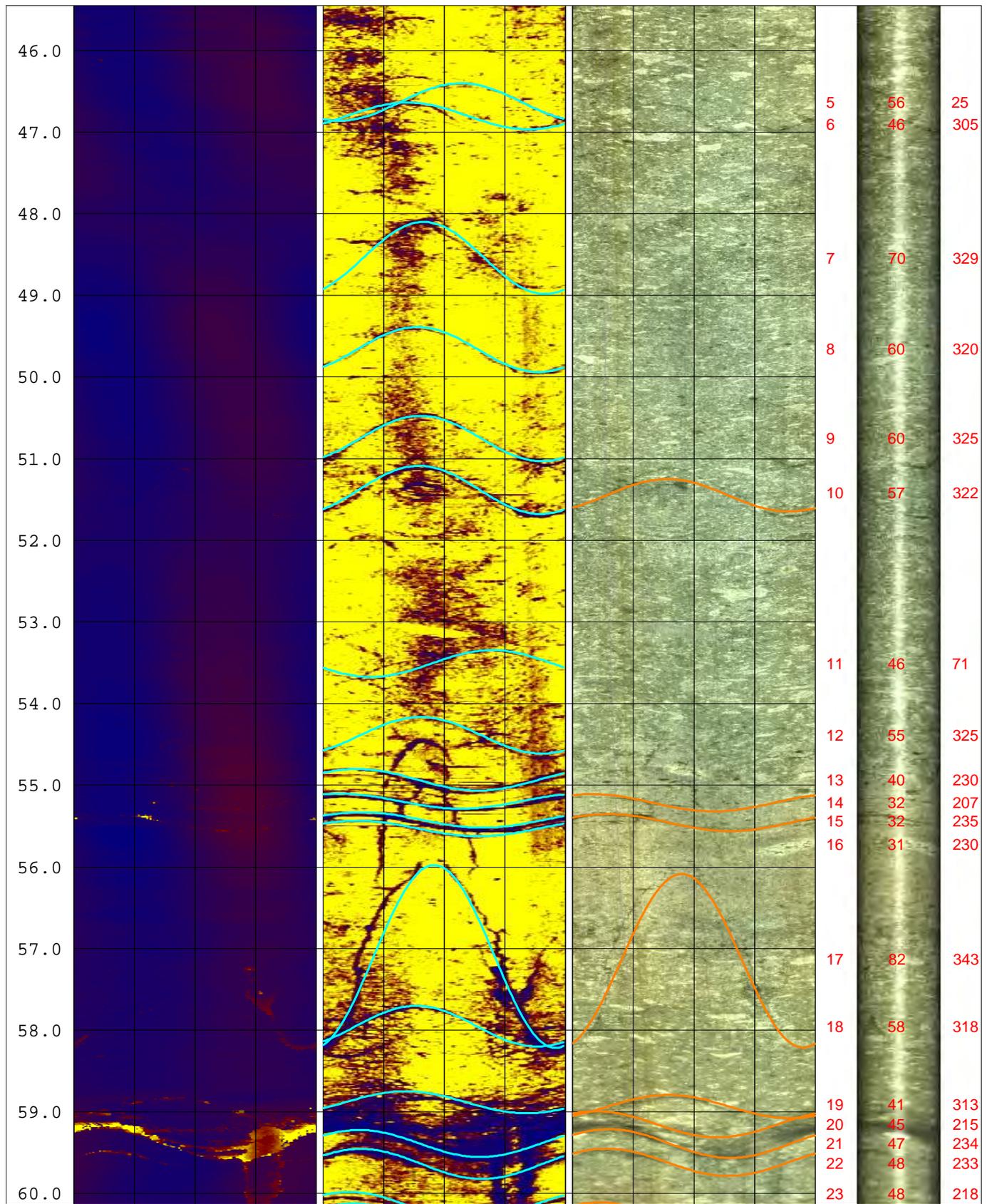
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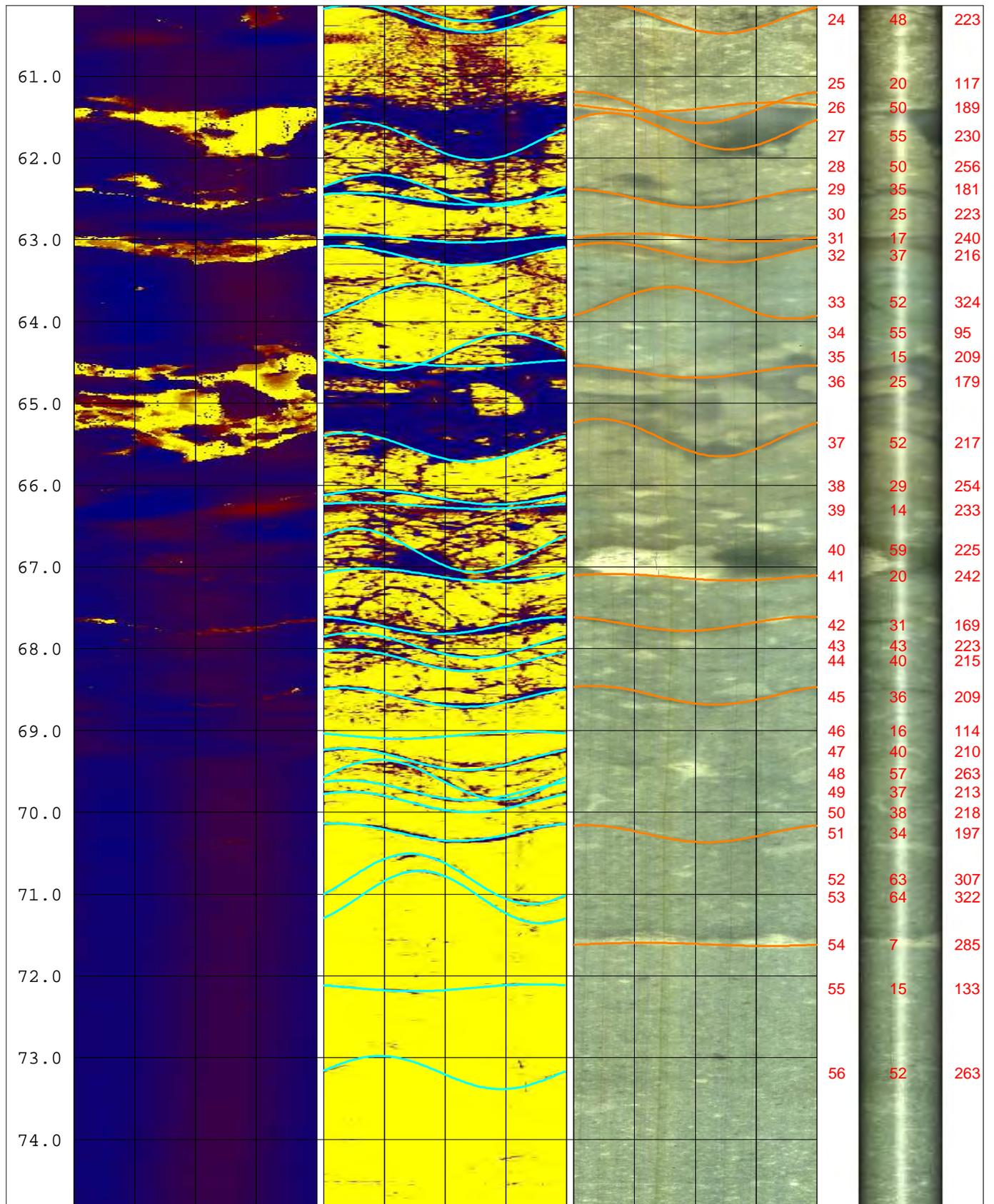
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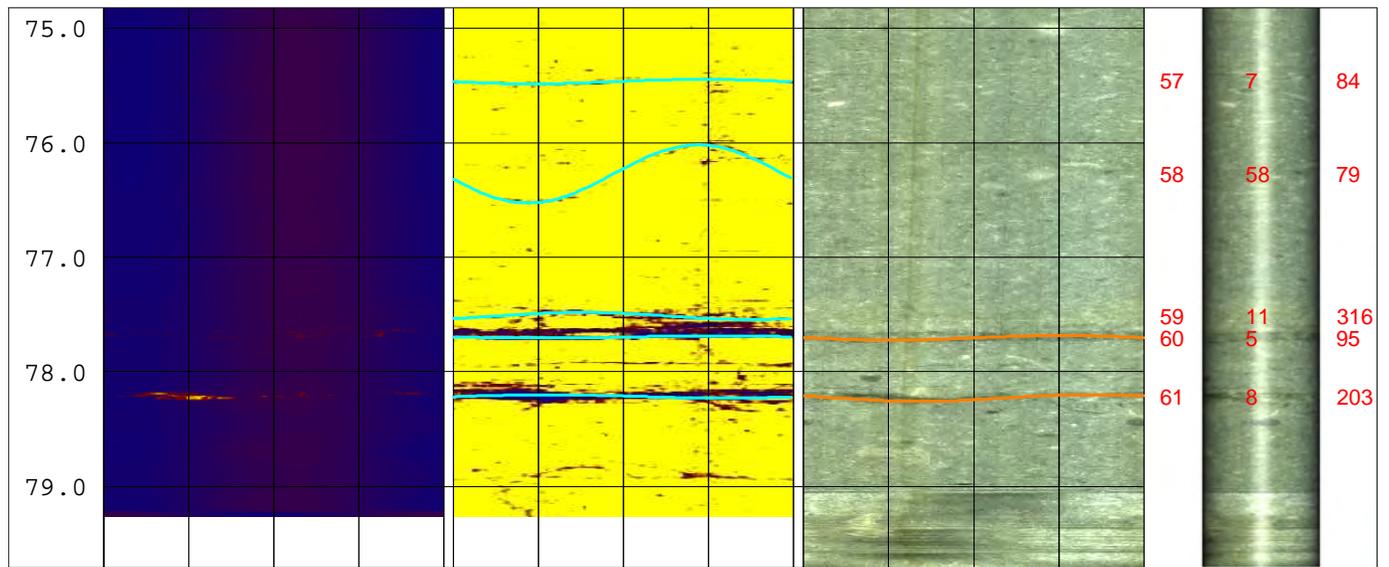
Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.











**APPENDIX C.4**  
**Slide Curve Bridge COBL® Logs**





Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project: I-90 Snoqualmie Pass East

Date Logged: June 11, 2009

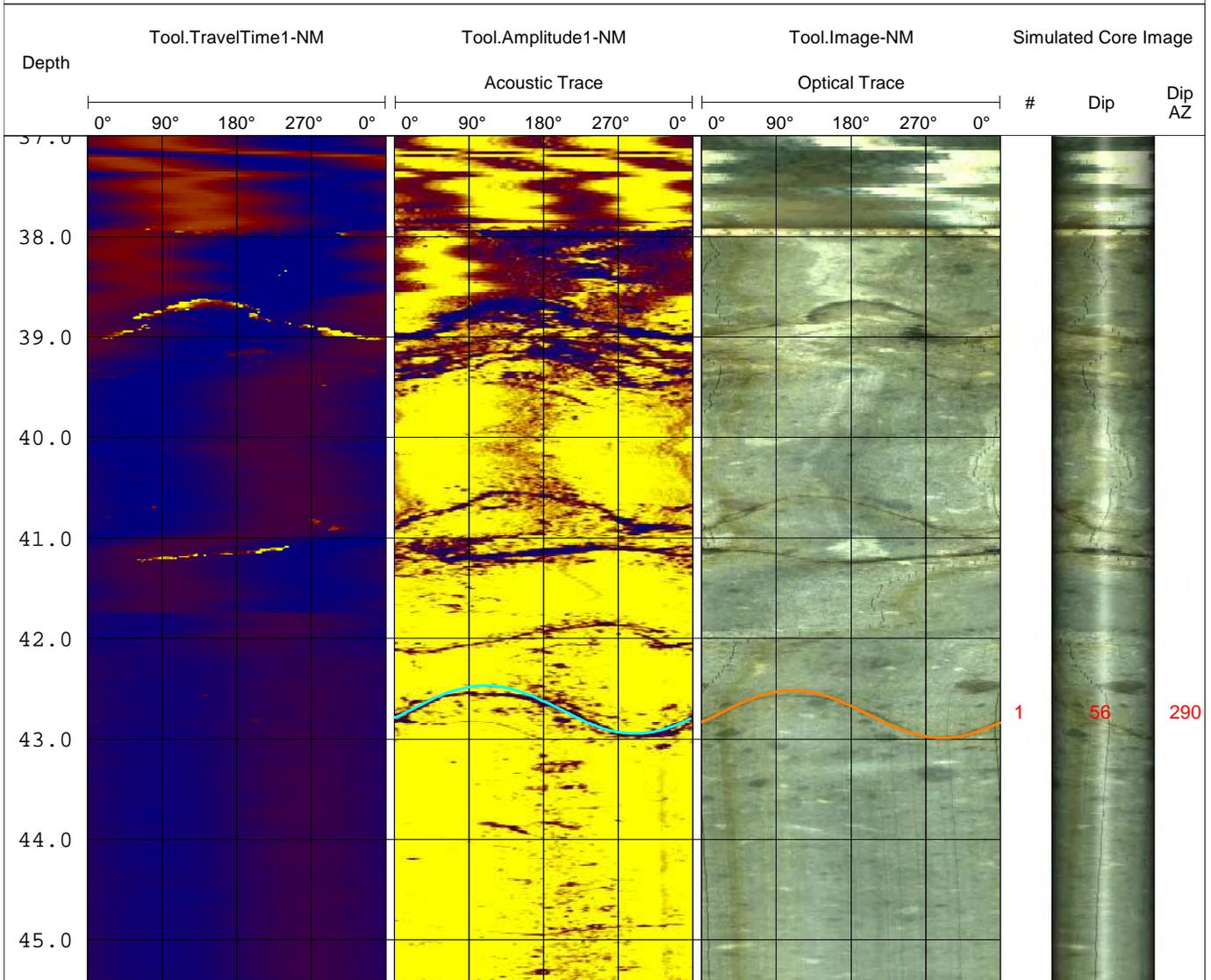
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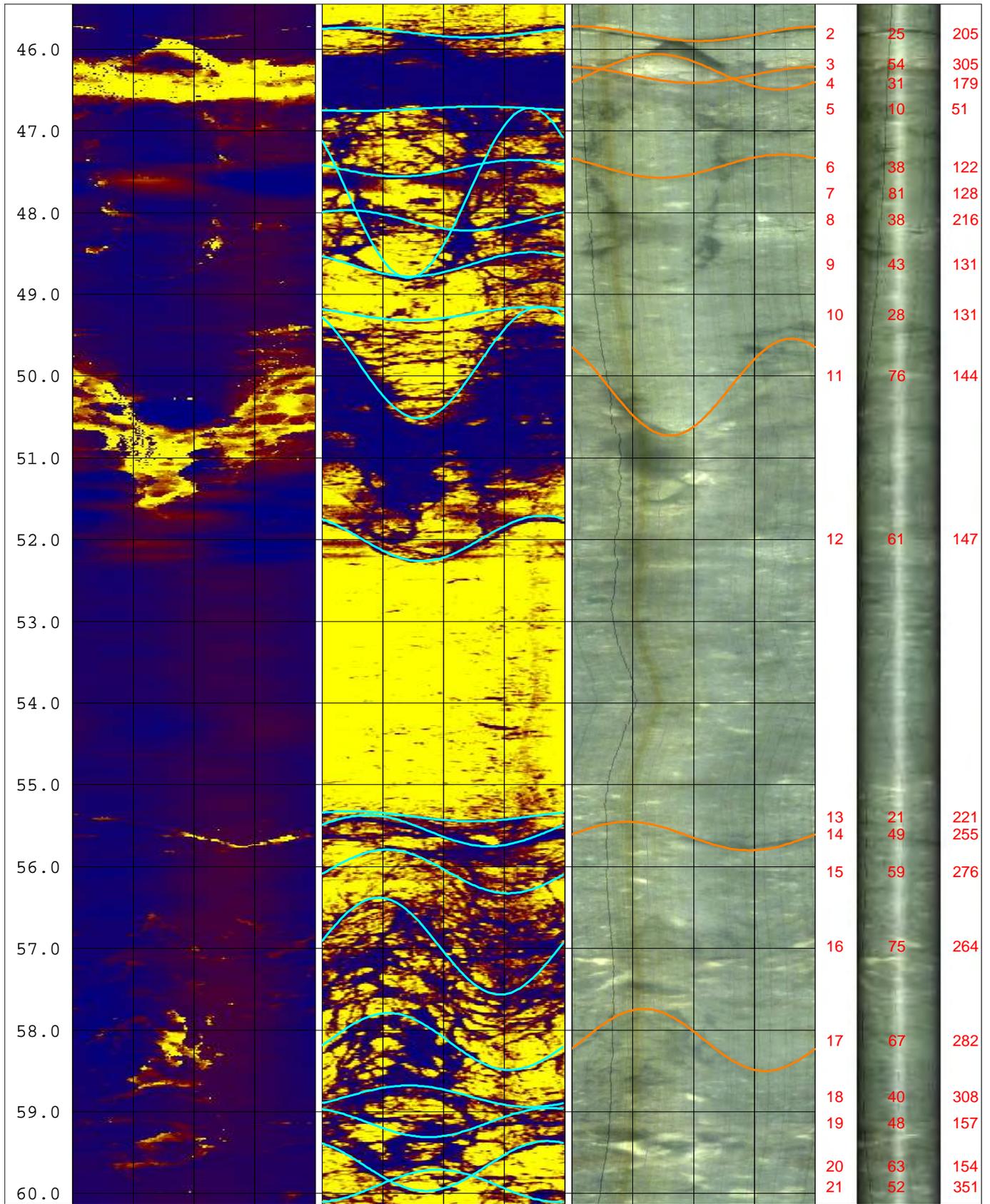
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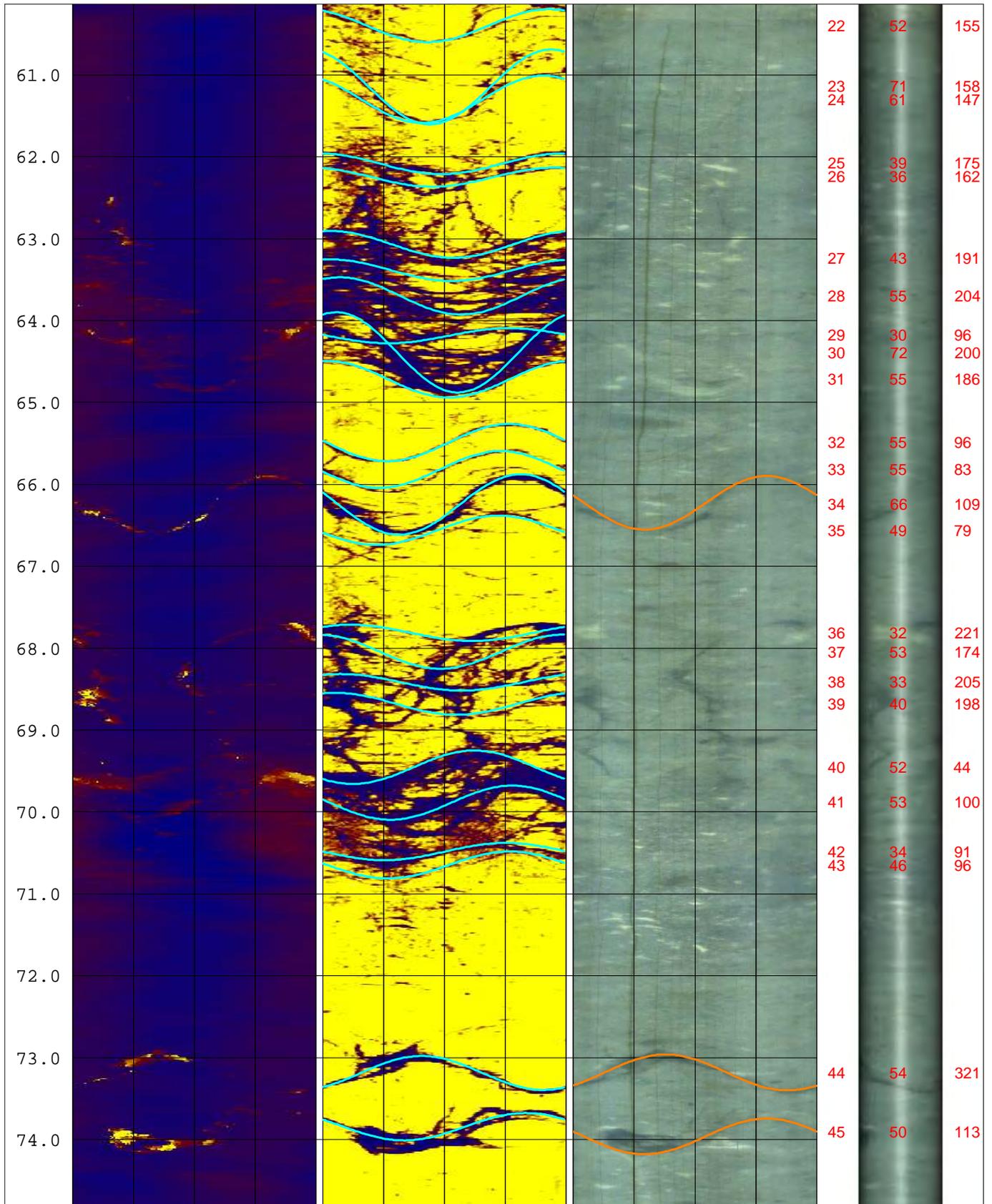
Logged By: F G Kruger

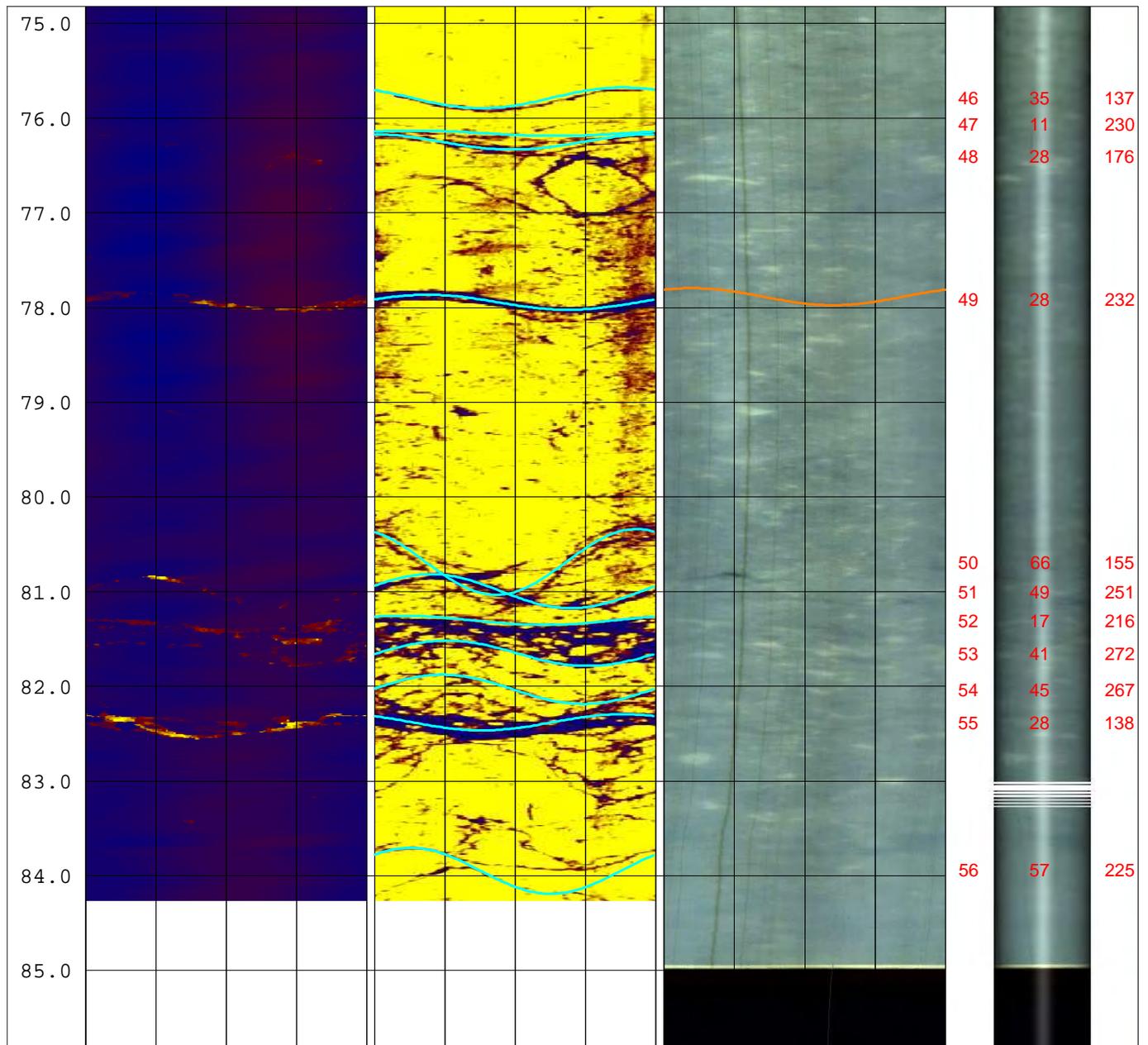
Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.











Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project:

Date Logged: October 11, 2009

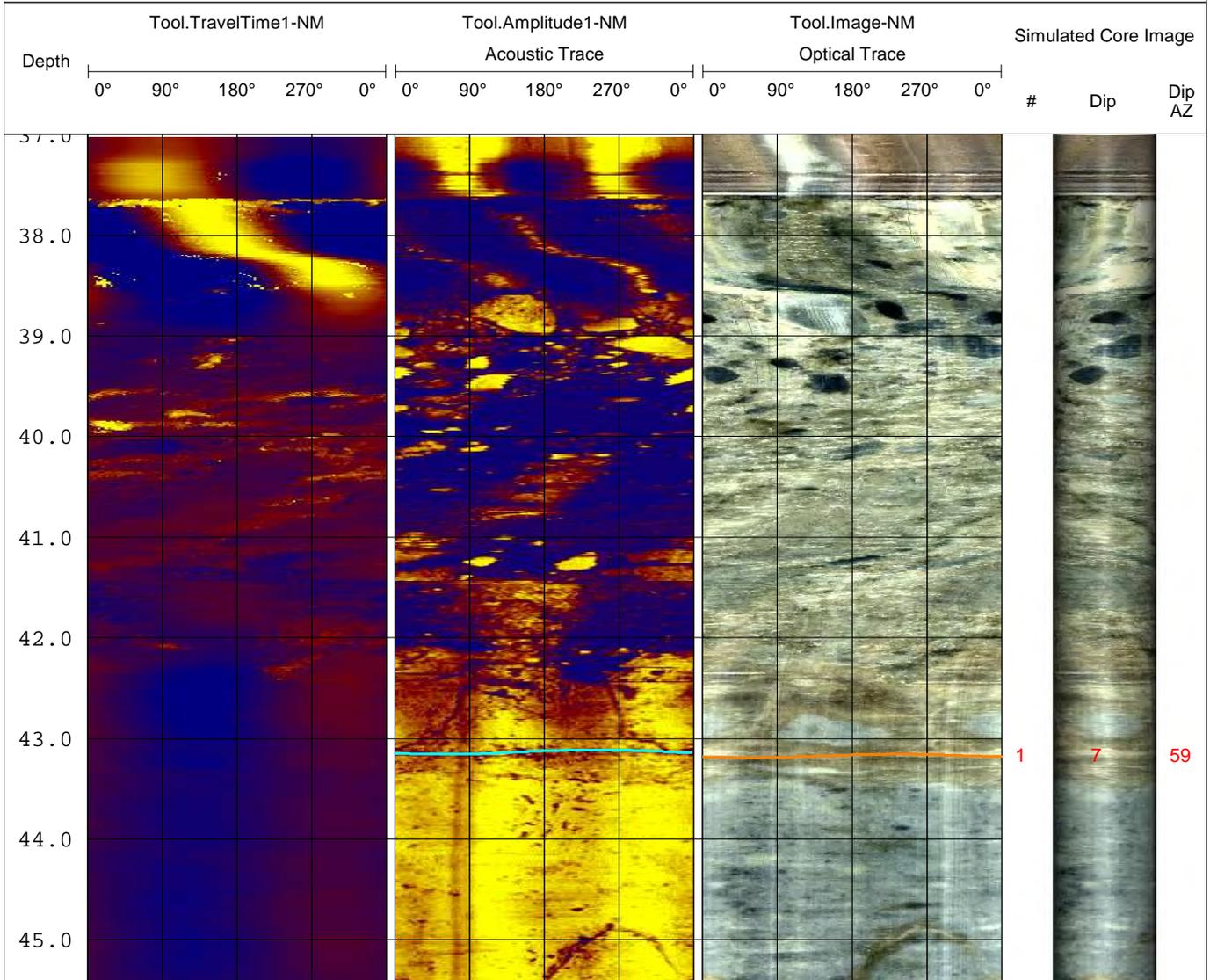
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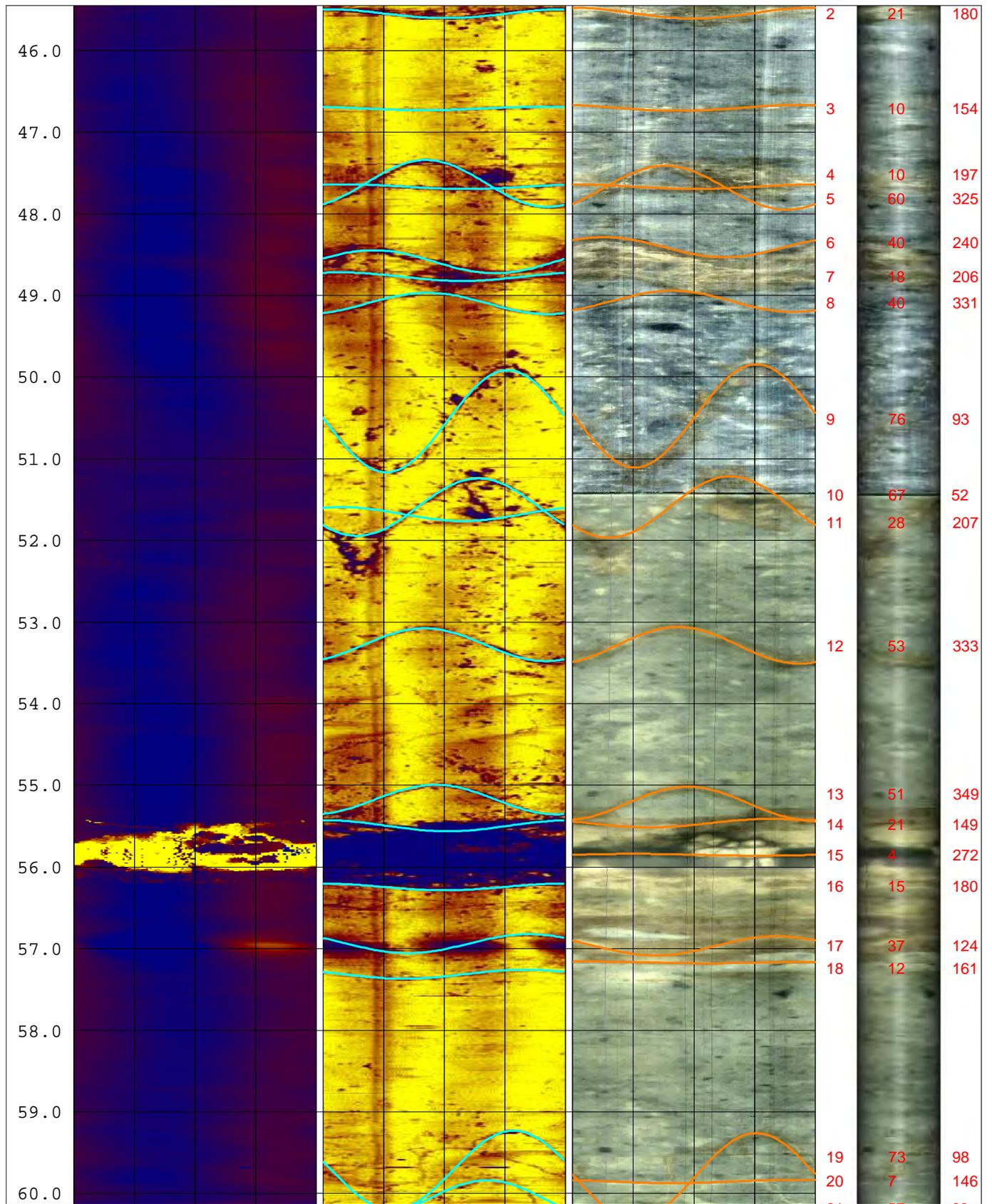
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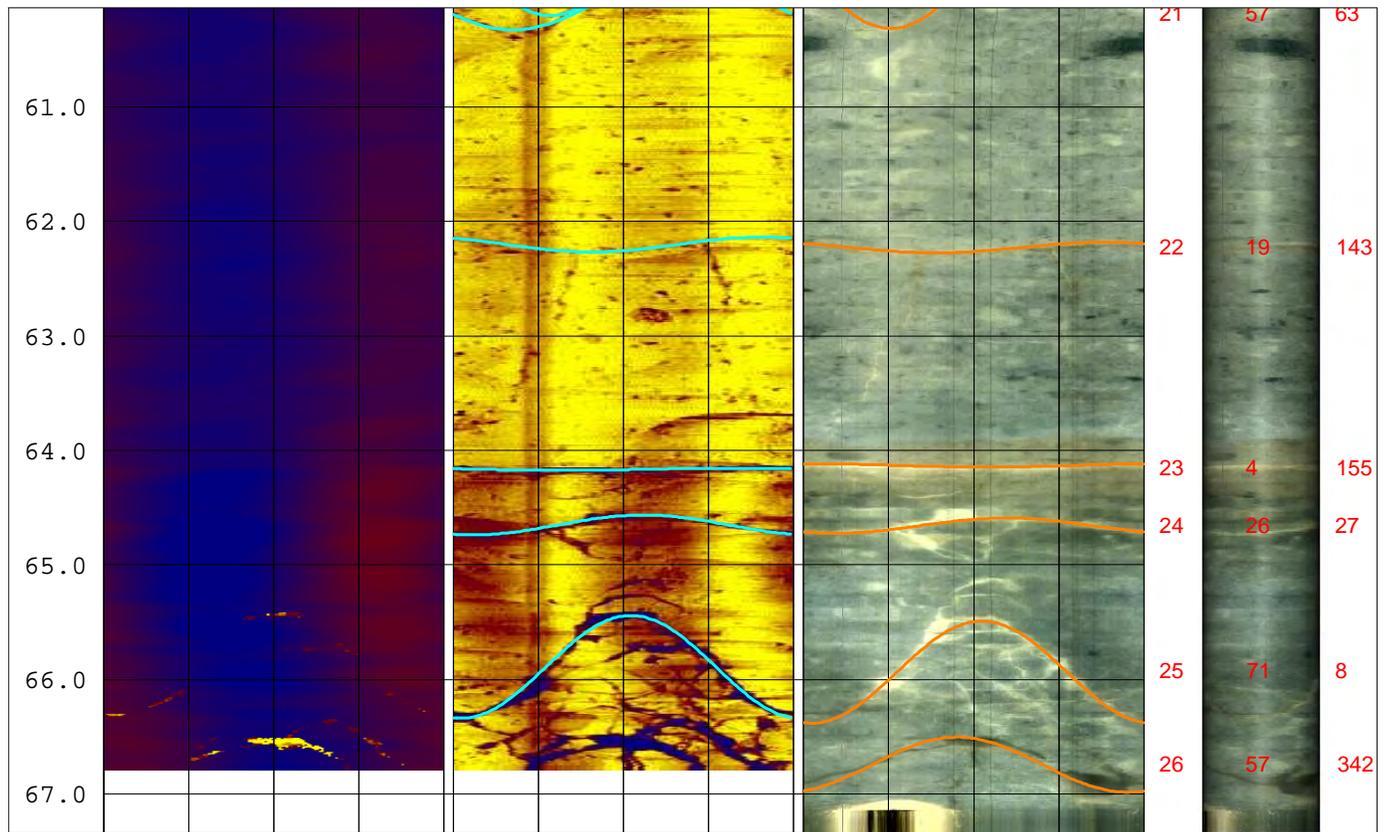
Logged By: Steve Wilson

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.









Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project:

Date Logged: October 14, 2009

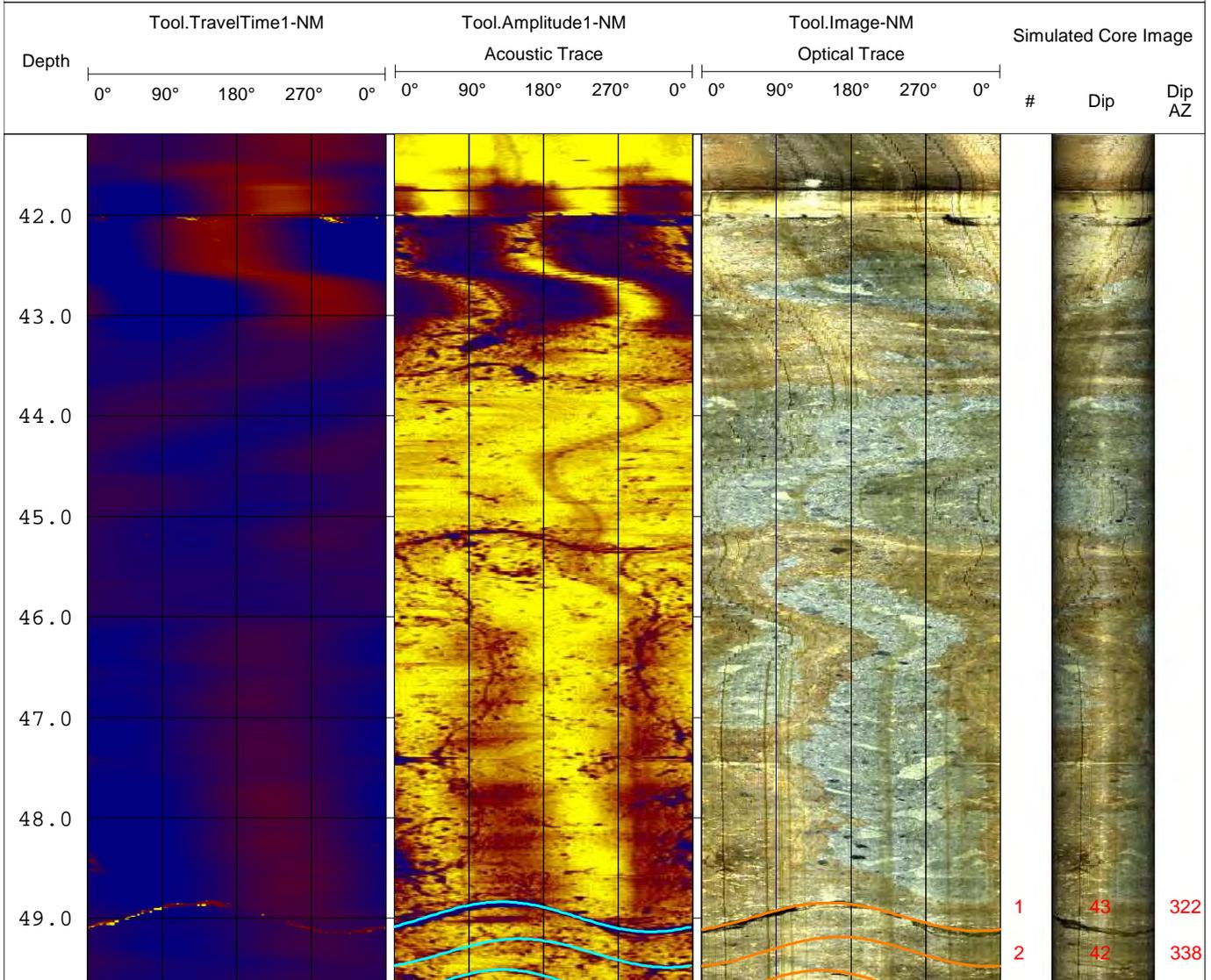
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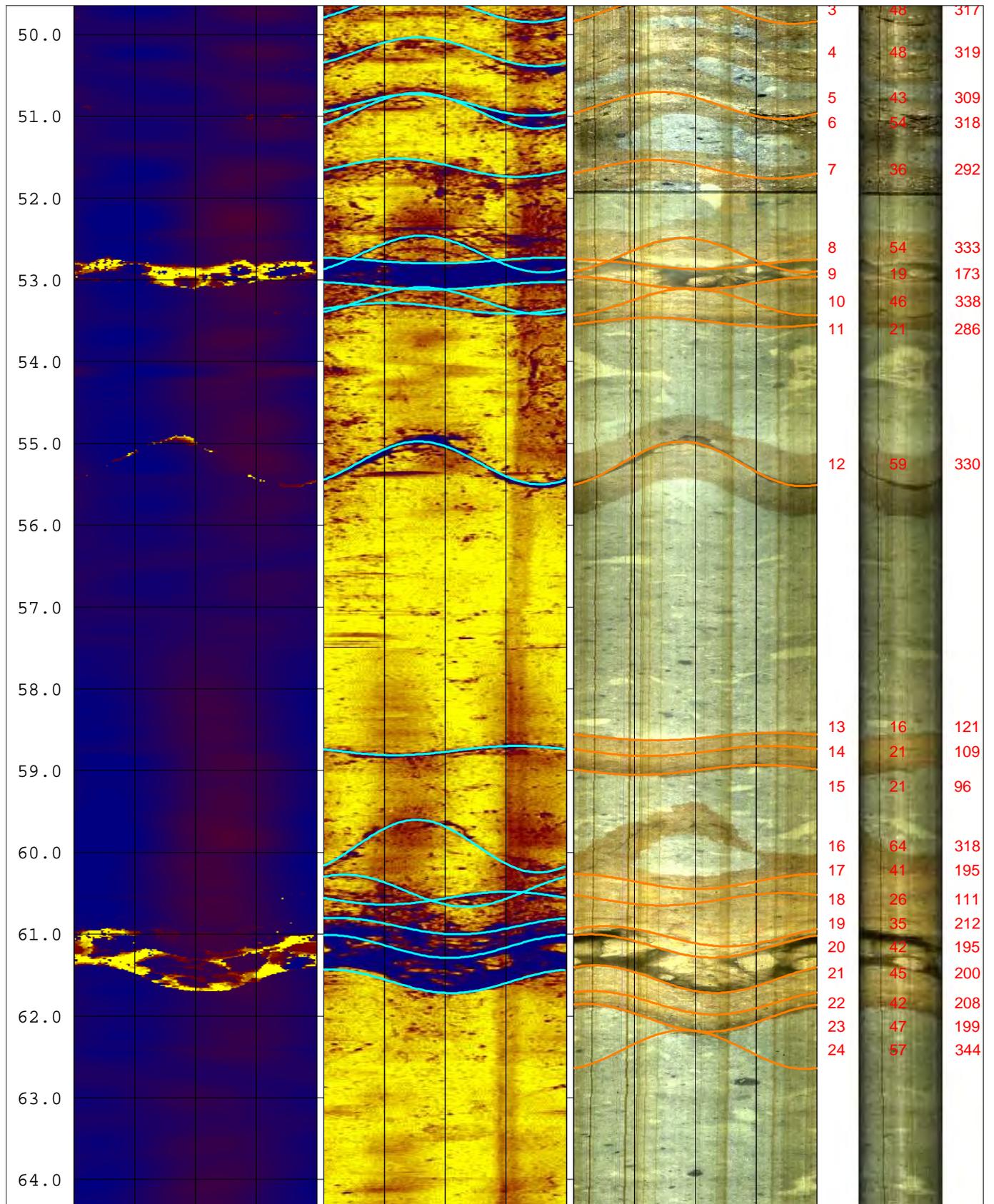
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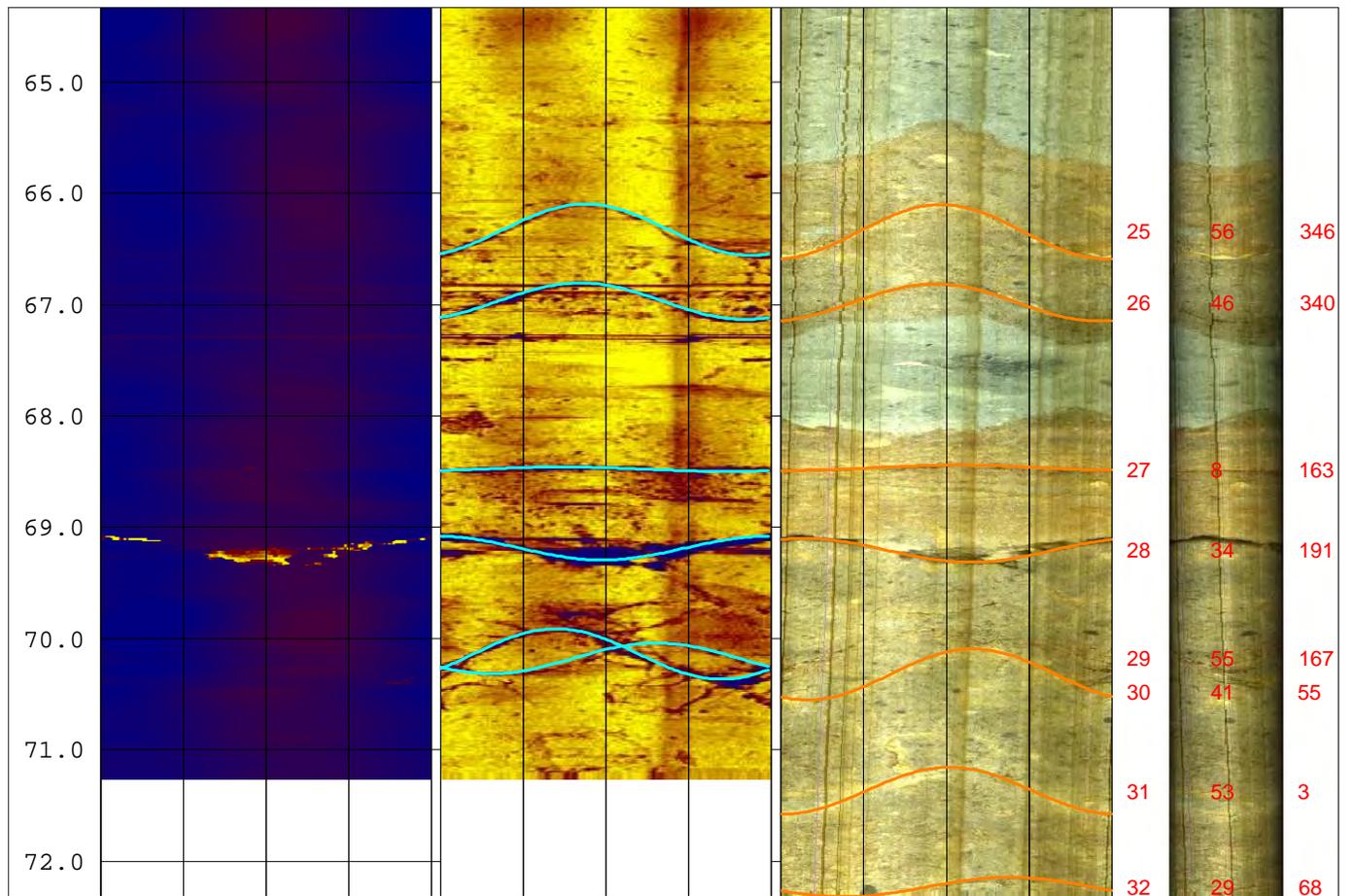
Logged By: F G Kruger

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.









Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project:

Date Logged: October 22, 2009

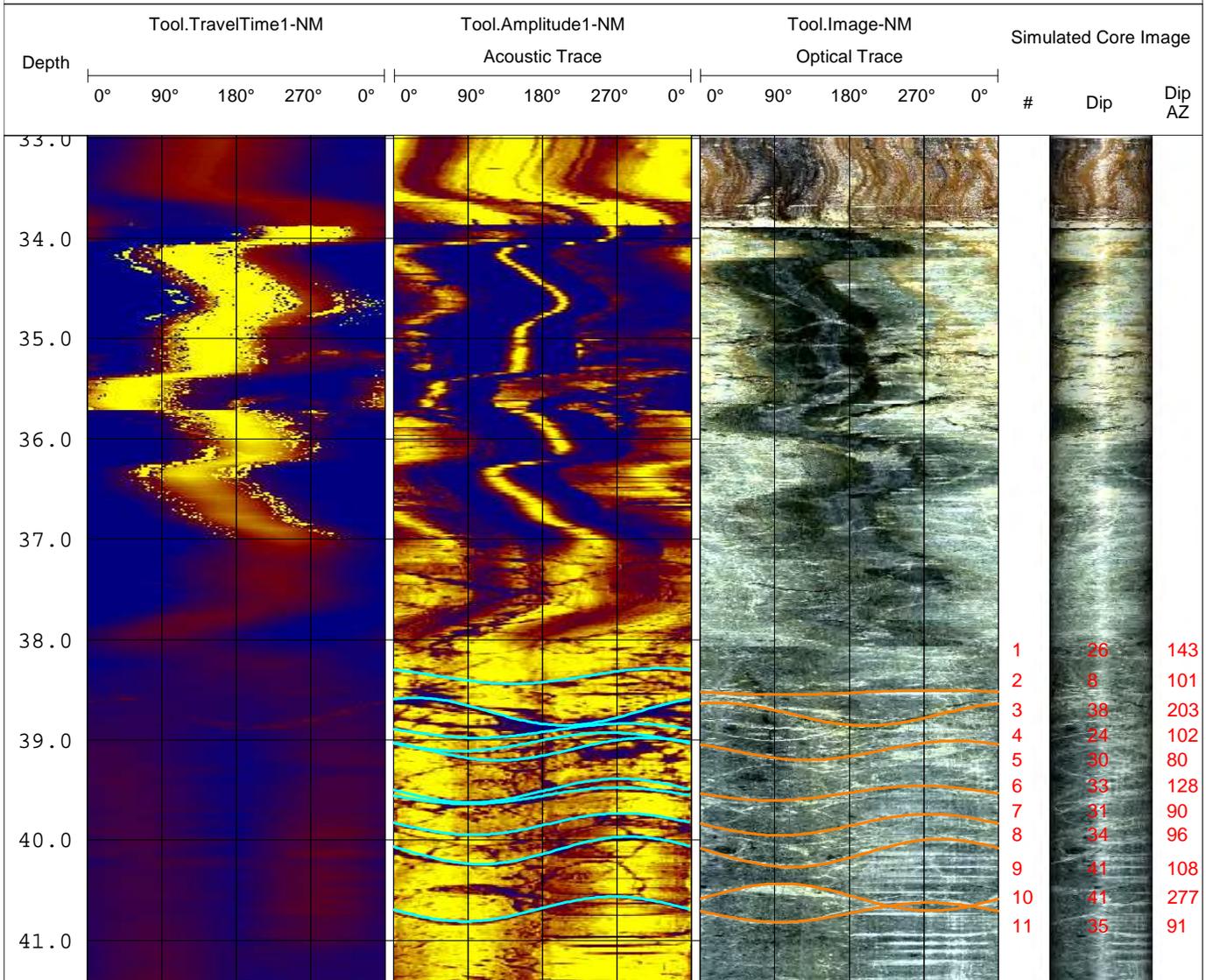
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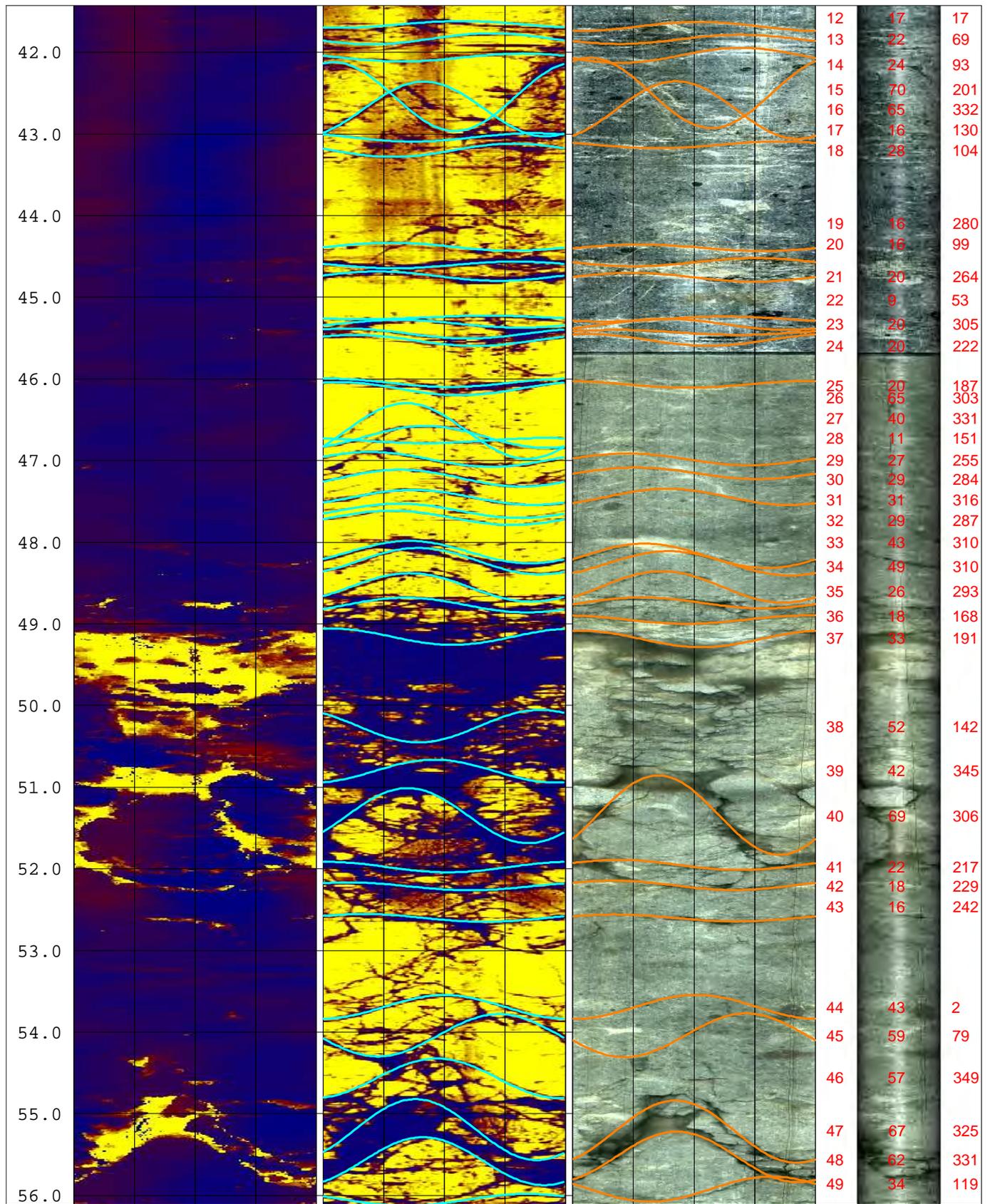
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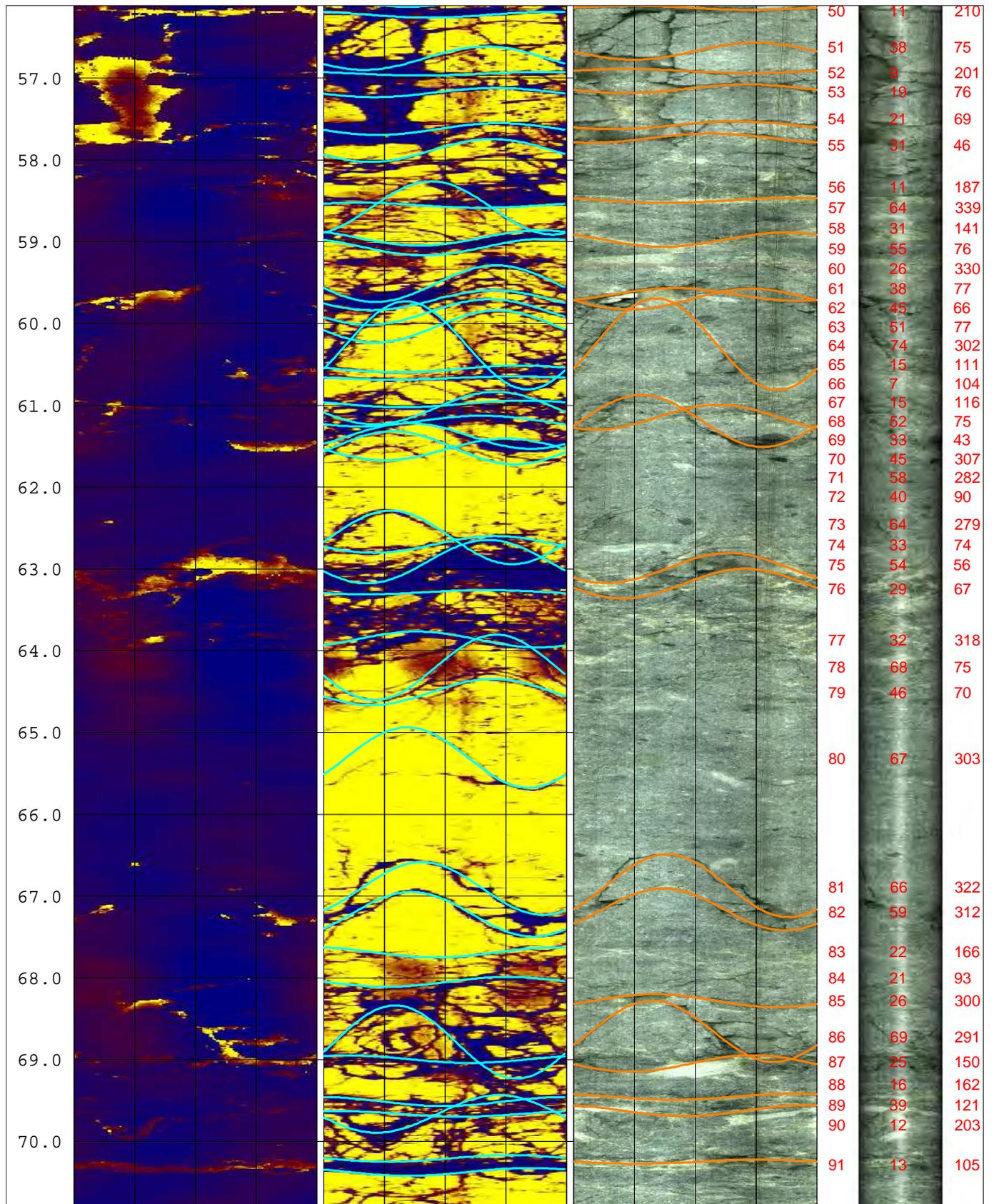
Logged By: Chris Mills

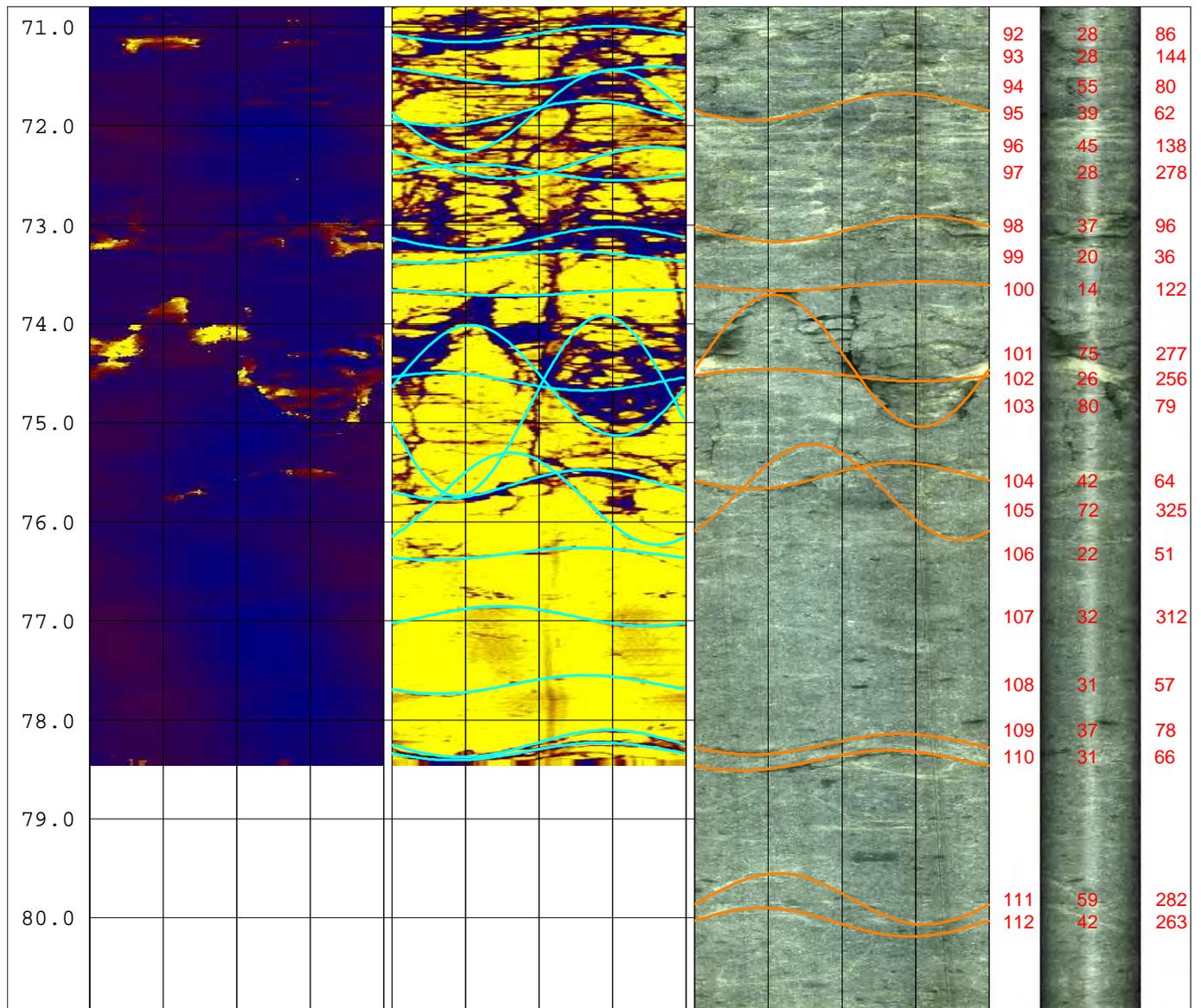
Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiever Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.











Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project:

Date Logged: October 24, 2009

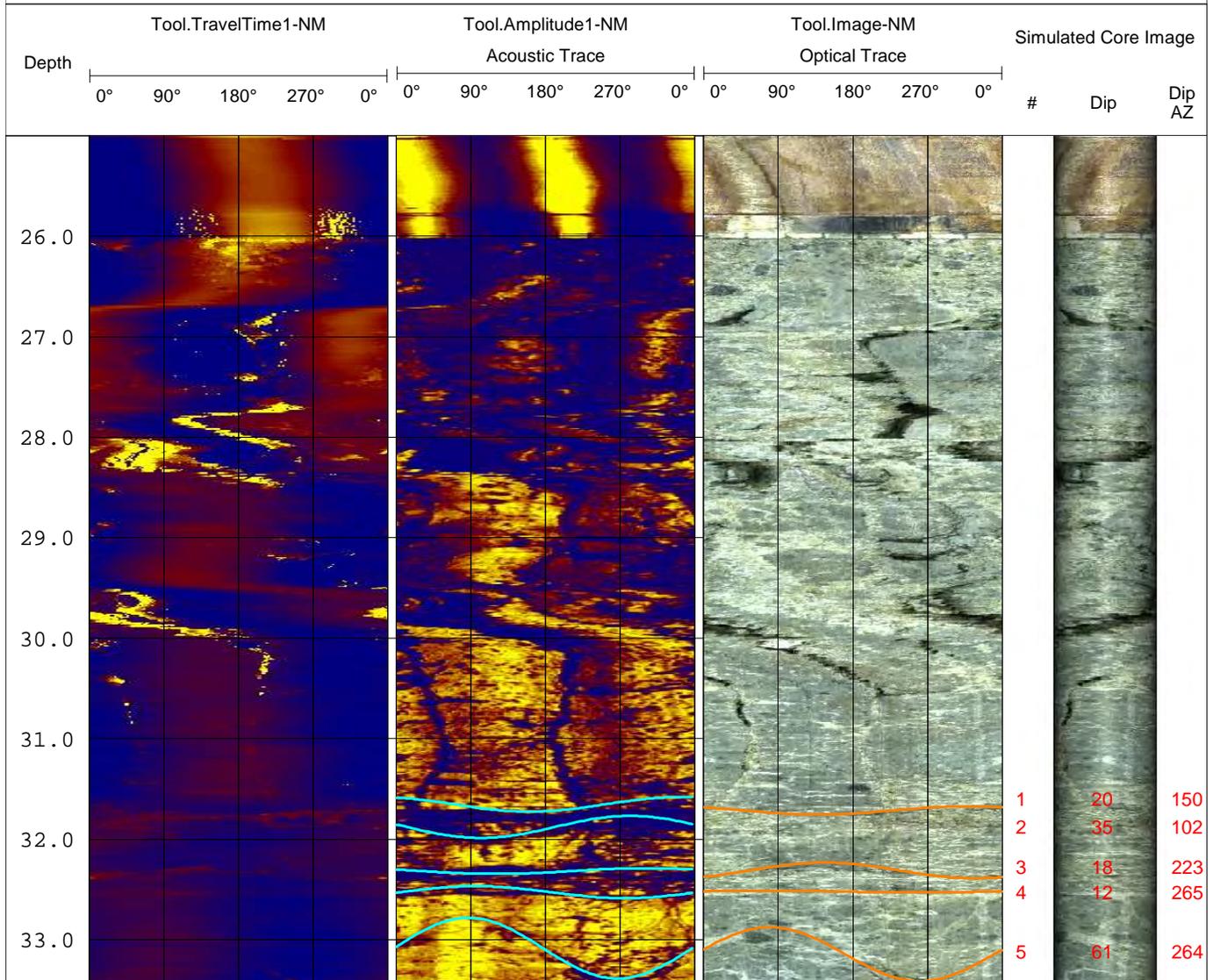
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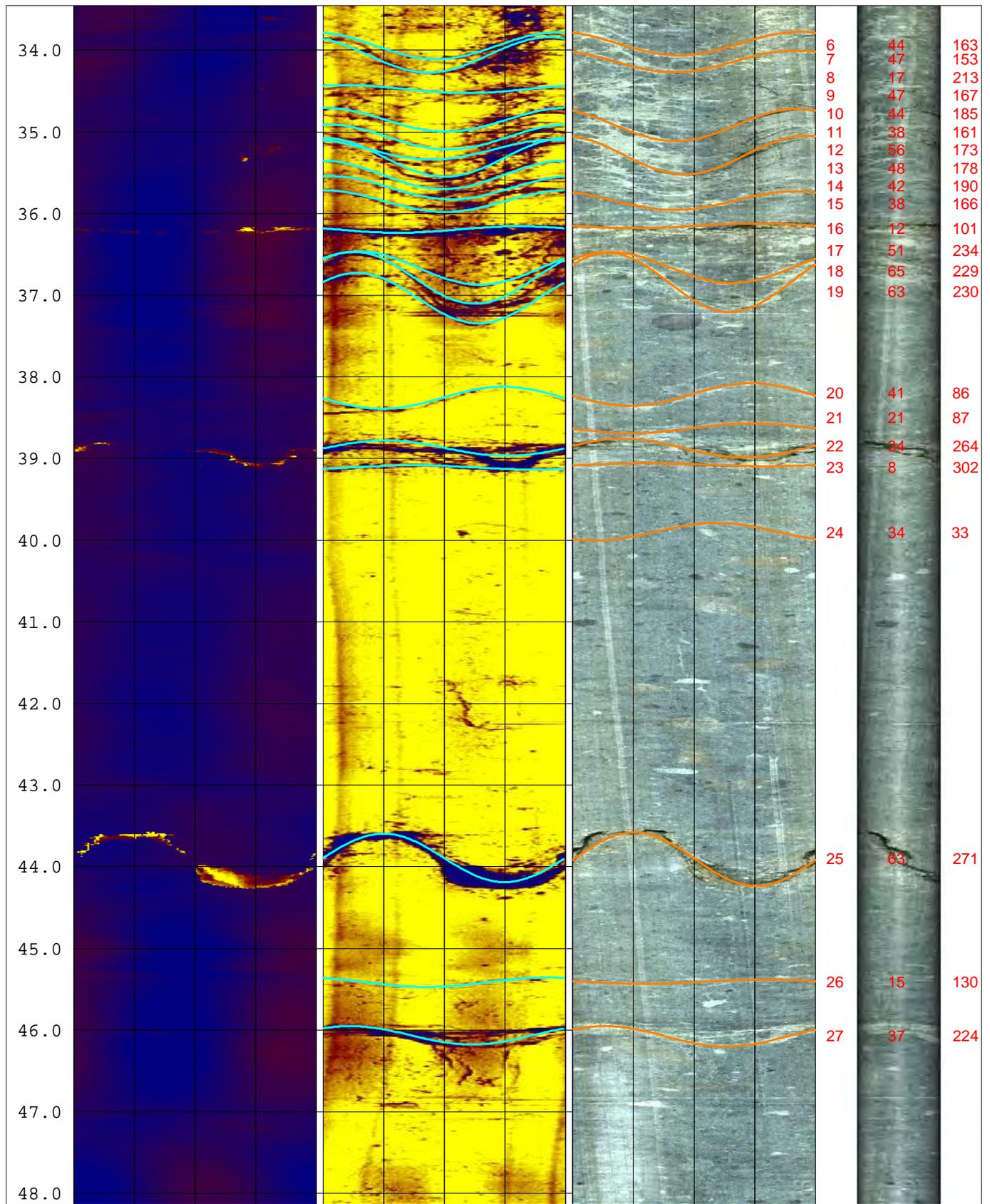
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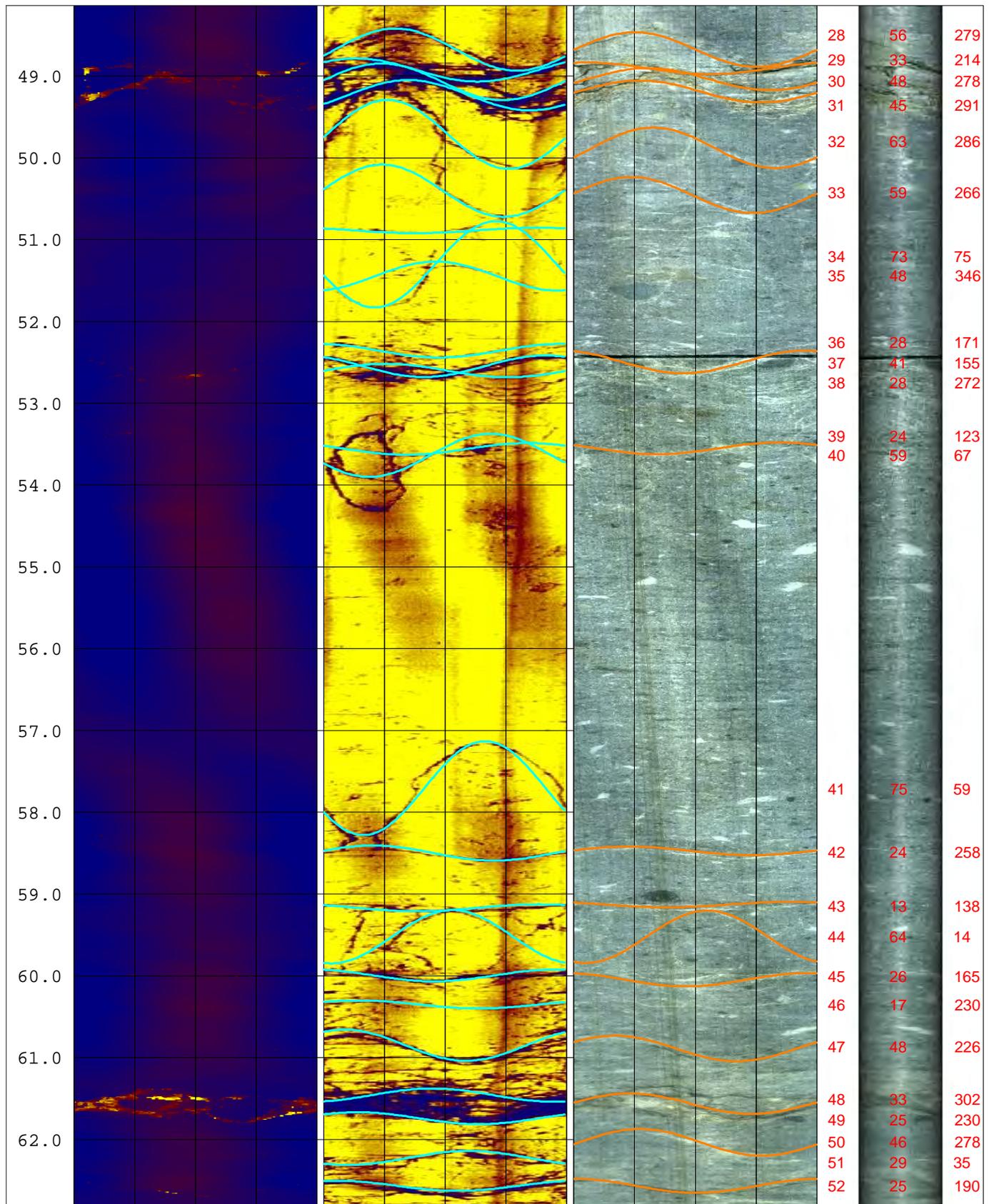
Logged By: Chris Mills

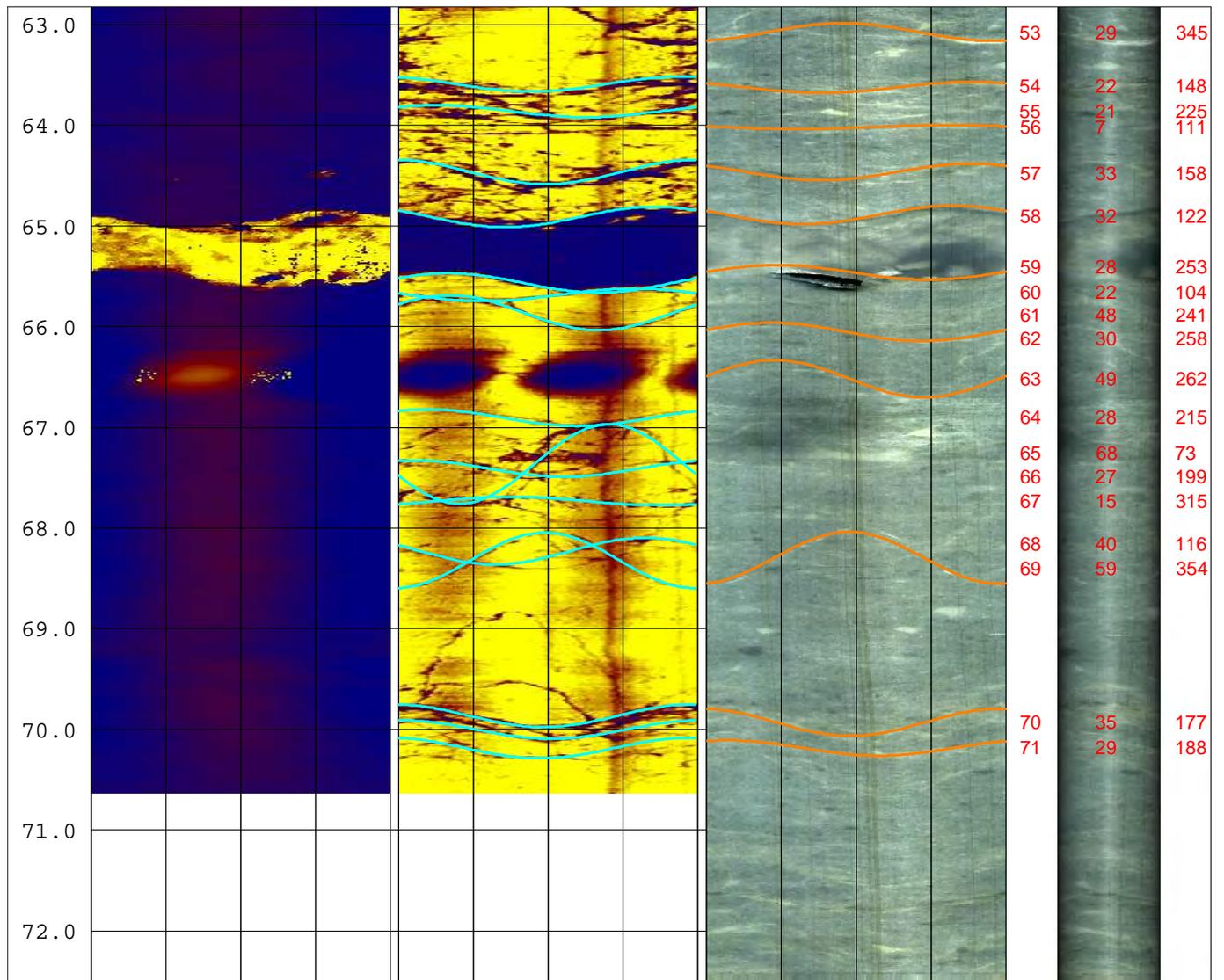
Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.











Company Washington State Dept. of Transportation

Location: Interstate 90 - Snoqualmie Pass

Project:

Date Logged: October 25, 2009

Borehole ID: SCB-30-09

File Name: SCB3009-1M.wcl

Logged By: Chris Mills

Notes:

- 1) Azimuths reference True North. Magnetic declination of +17 degrees applied.
- 2) 3x Horizontal Exaggeration on Televiewer Images.
- 3) Dip values presented in degrees from horizontal.
- 4) Dip direction presented in degrees of azimuth.
- 5) Simulated Core Image is viewed from north, east on left, west on right.

